

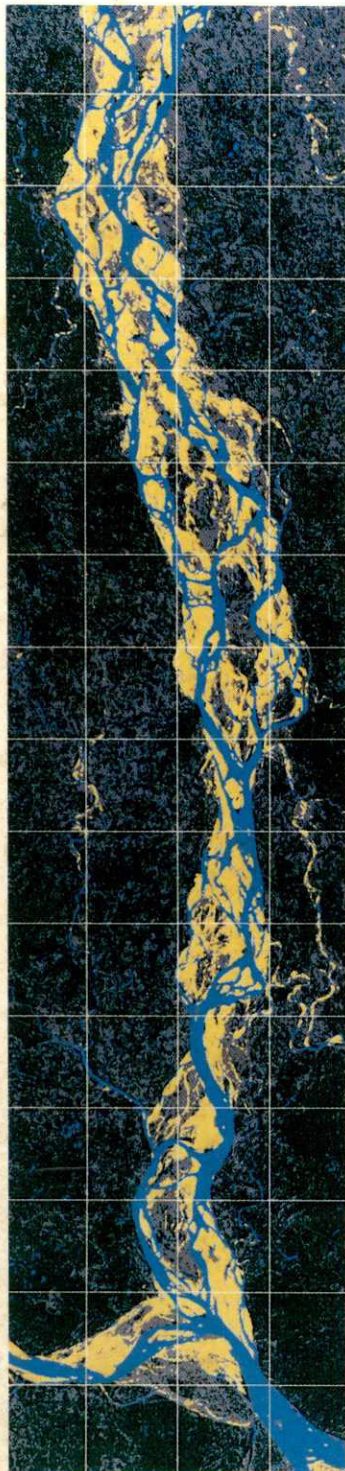
Call - 823
FAP-21/22

GOVERNMENT OF PEOPLE'S REPUBLIC OF BANGLADESH
MINISTRY OF IRRIGATION, WATER DEVELOPMENT AND FLOOD CONTROL
FLOOD PLAN COORDINATION ORGANIZATION

KREDITANSTALT FÜR
WIEDERAUFBAU (KfW)



CAISSE FRANCAISE DE
DEVELOPPEMENT (CFD)



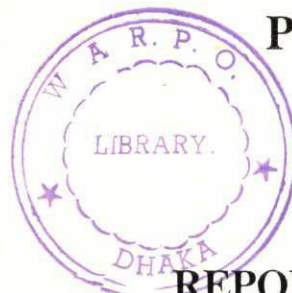
(49)

BANK PROTECTION AND
RIVER TRAINING (AFPM)
PILOT PROJECT
FAP 21/22

BN - 676
A - 823 (1)



TEST
AND
IMPLEMENTATION
PHASE



REPORT ON
MONITORING AND ADAPTATION
AT
KAMARJANI TEST SITE

INTERIM REPORT (ANNEXES)

MARCH 1996



CONSULTING CONSORTIUM FAP 21/22

RHEIN-RUHR ING.-GES.MBH, DORTMUND/GERMANY

COMPAGNIE NATIONALE DU RHONE, LYON/FRANCE
PROF.DR. LACKNER&PARTNERS, BREMEN/GERMANY
DELFT HYDRAULICS, DELFT/NETHERLANDS

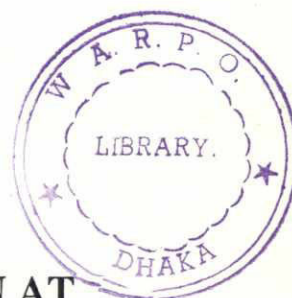
In association with:

BANGLADESH ENGINEERING &
TECHNOLOGICAL SERVICES LTD.(BETS)
DESH UPODESH LIMITED (DUL)

2

**BANK PROTECTION AND RIVER TRAINING
(AFPM) PILOT PROJECT
FAP 21/22**

TEST AND IMPLEMENTATION PHASE



**REPORT ON MONITORING AND ADAPTATION AT
KAMARJANI TEST SITE**

MFV-2390
24-02
C-1

A-72

INTERIM REPORT (ANNEXES)

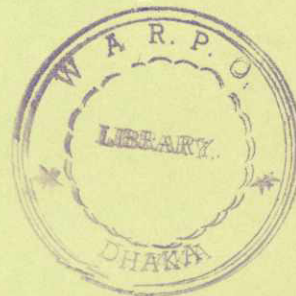
MARCH 1996

BANK PROTECTION AND RIVER TRAINING/AFPM PILOT PROJECT
FAP 21/22

REPORT ON MONITORING AND ADAPTATION
AT KAMARJANI TEST SITE

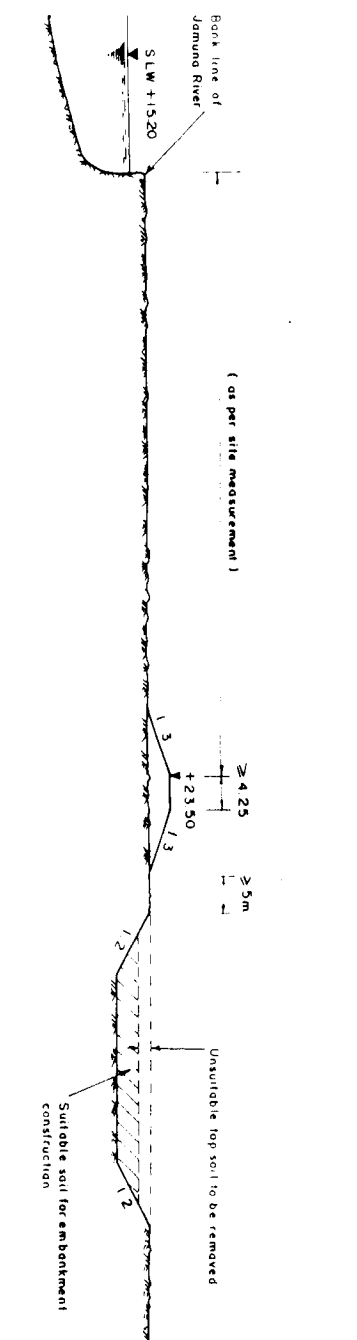
List of Annexes

Annex A:	As Built Drawings
Annex B:	(Reserved for Final Report)
Annex C:	(Reserved for Final Report)
Annex D:	(Reserved for Final Report)
Annex E:	Model Tests - Photographs
Annex F:	Detailed Description of Works
Annex G:	Detailed Construction Time Schedule
Annex H:	Photographs



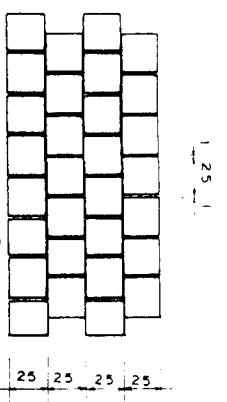
ANNEX A

As Built Drawings



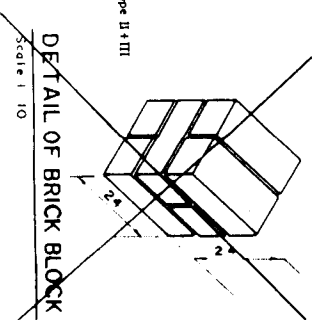
RECOMMENDED CROSS-SECTION OF EMBANKMENT AT TEST SITE 1

Scale 1 500



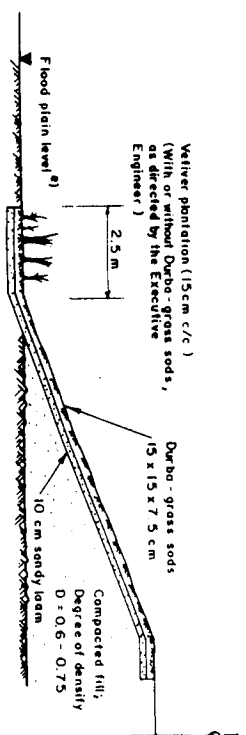
DETAIL 'D'

Scale 1 20 (measurements in cm)



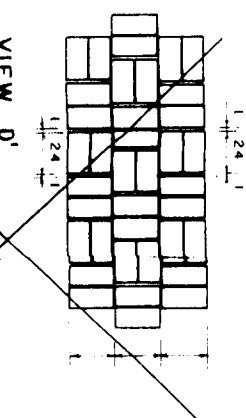
SECTION b - b

~~Scale 1 10~~



VIEW D'
LAYING PATTERN OF

Scale 1 : 20



**BWDB - EMBANKMENT
REVETMENT DETAILS**


4) or as decided at Site by the Employer

[illegible]

DRAWING PHOTOREDUCTION BY 50 %

GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH
MINISTRY OF IRRIGATION WATER DEVELOPMENT & FLOOD CONTROL
FLOOD PLAN COORDINATION ORGANIZATION (FPCO)

BANK PROTECTION PILOT PROJECT FAP - 21



CONSULTING CONSORTIUM

CONSULTING CONSORTIUM

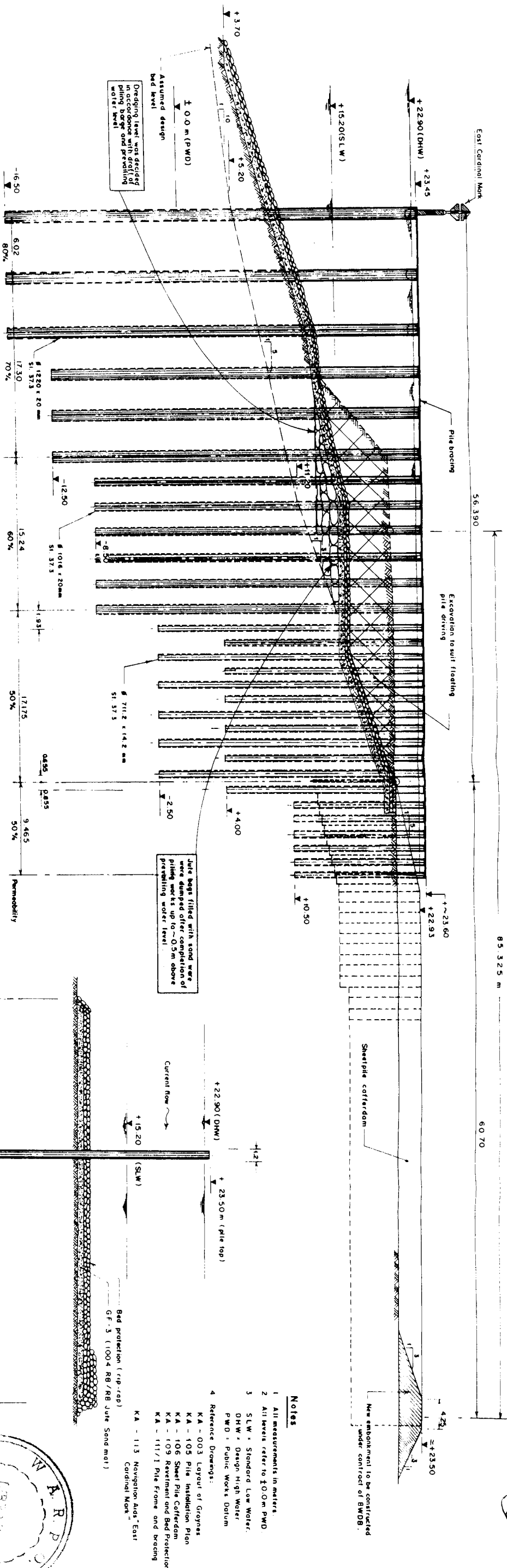
BREXIT RING INC. (S.S. 1981), DORTMUND/GERMANY
COMING FROM AUTOMATA DU BORME, L'YVINGE
FOR IN LACKENBACH, REIMS/GERMANY
DEUT. BUNDELS. RECHENFABRIK
DEUT. BUNDELS. RECHENFABRIK

**BAU-ANFANG ENGINEERING &
TECHNOLOGICAL SERVICES LTD (NET
FROM UNIKLIN LIMITED LTD)**

in association with

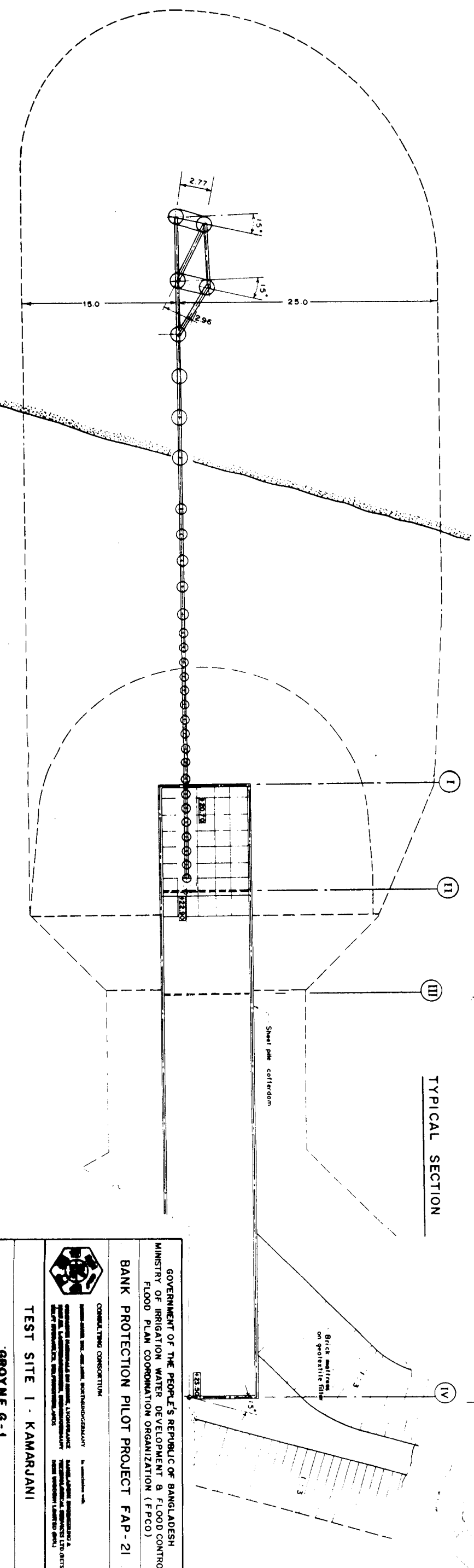
TEST SITE 1 - KAMARJANI

**BWDB - EMBANKMENT
REVEIMENT DETAILS**



ELEVATION

TYPICAL SECTION



PLAN

DRAWING PHOTO REDUCED BY 50

REV.	DATE	NAME	DESCRIPTION	APPROVED	DATE	NAME	SCALE	REVISION
3	25.01	ABDUL	AS BUILT DRAWING				1:200	
2	24.04	ABDUL	RIVER BED LEVEL					
1	20.04	ABDUL	PILE LAYOUT, GENERAL, PLATFORM					

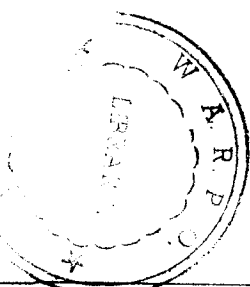


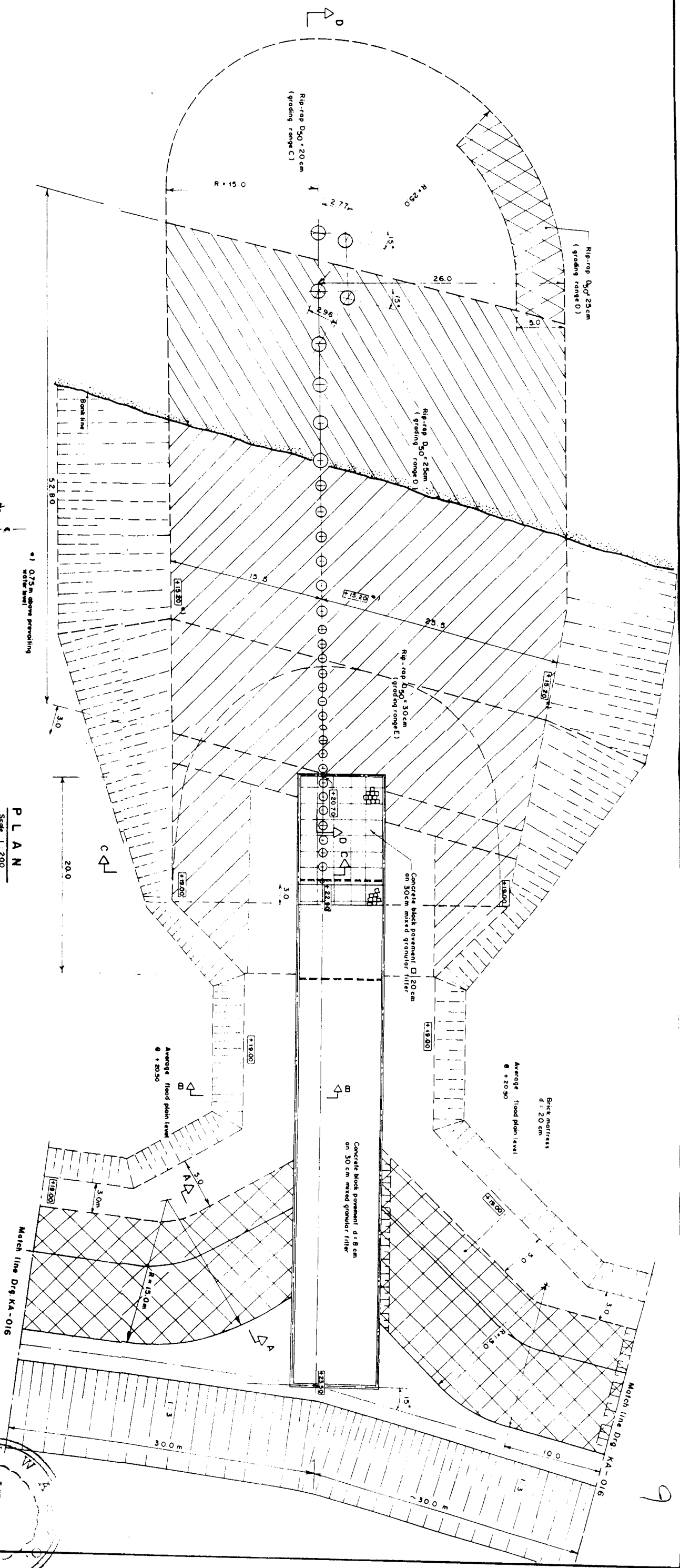
GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH
MINISTRY OF IRRIGATION WATER DEVELOPMENT & FLOOD CONTROL
FLOOD PLAN COORDINATION ORGANIZATION (FPCO)

BANK PROTECTION PILOT PROJECT FAP-21

TEST SITE I - KAMARJANI

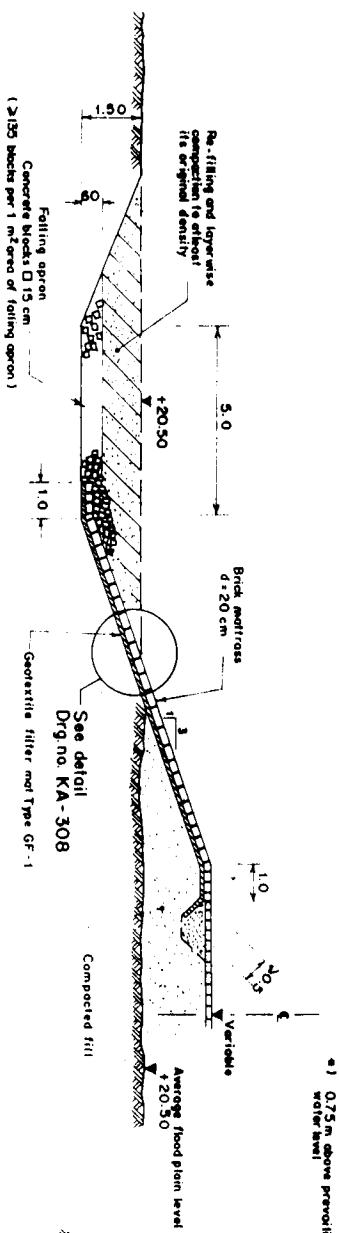
ØROYNE G-1
GENERAL ARRANGEMENT
ELEVATION, PLAN, SECTION





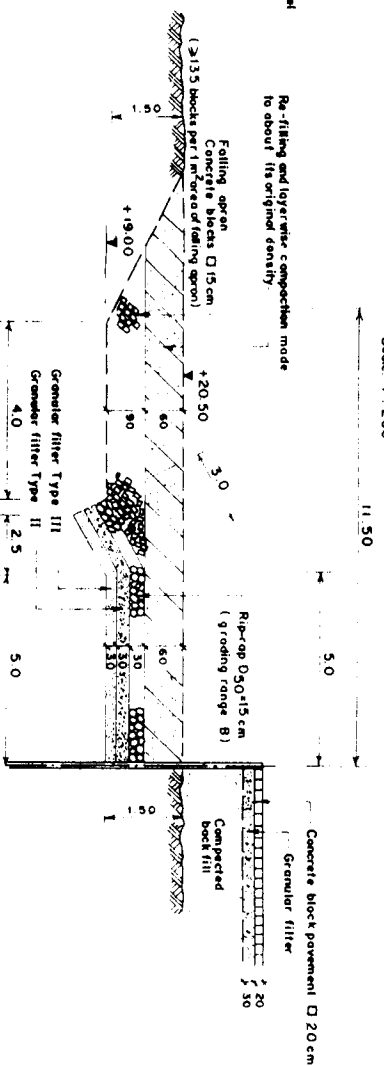
SECTION A-A

Scale: 1 : 100



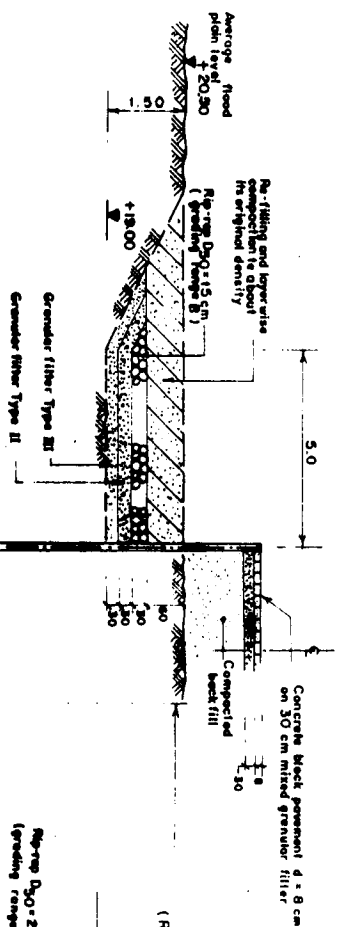
SECTION C-C

Scale : 1 : 100



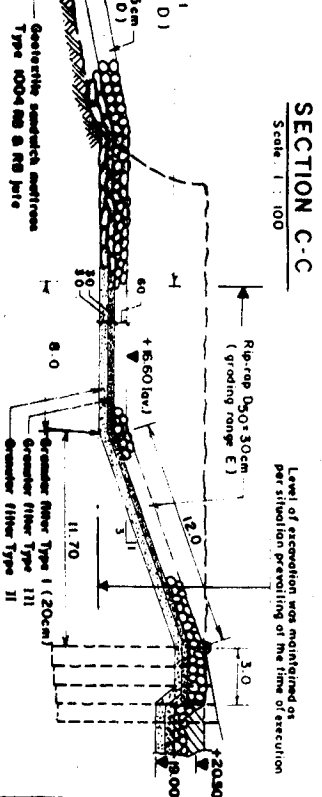
SECTION B-B

Scale. 1 : 100



SECTION D-D

Scale: 1 : 100



DRAWING PHOTOREDUCED BY 50 %

REV.	DATE	NAME	DESCRIPTION	APPROVED	APPROVED	NAME	DATE	SCALE:	DRAWING NO.	REVISION
3	2/3/95	ANONWA	AS BUILT DRAWINGS	<i>[Signature]</i>				1:100, 1:200		
2	10/2/94	ANONWA	LEVELS	<i>[Signature]</i>		ANONWA	07-02-95			
1	30/9/94	ANONWA	PILE LAYOUT	<i>[Signature]</i>		<i>[Signature]</i>	19-01-94		KA - 109	3

TEST SITE I - KAMARJANI

GROYNE G-1

REVEIMENT AND BED PROTECTION PLAN, DETAILED SECTIONS

BANK PROTECTION PILOT PROJECT FAP-21

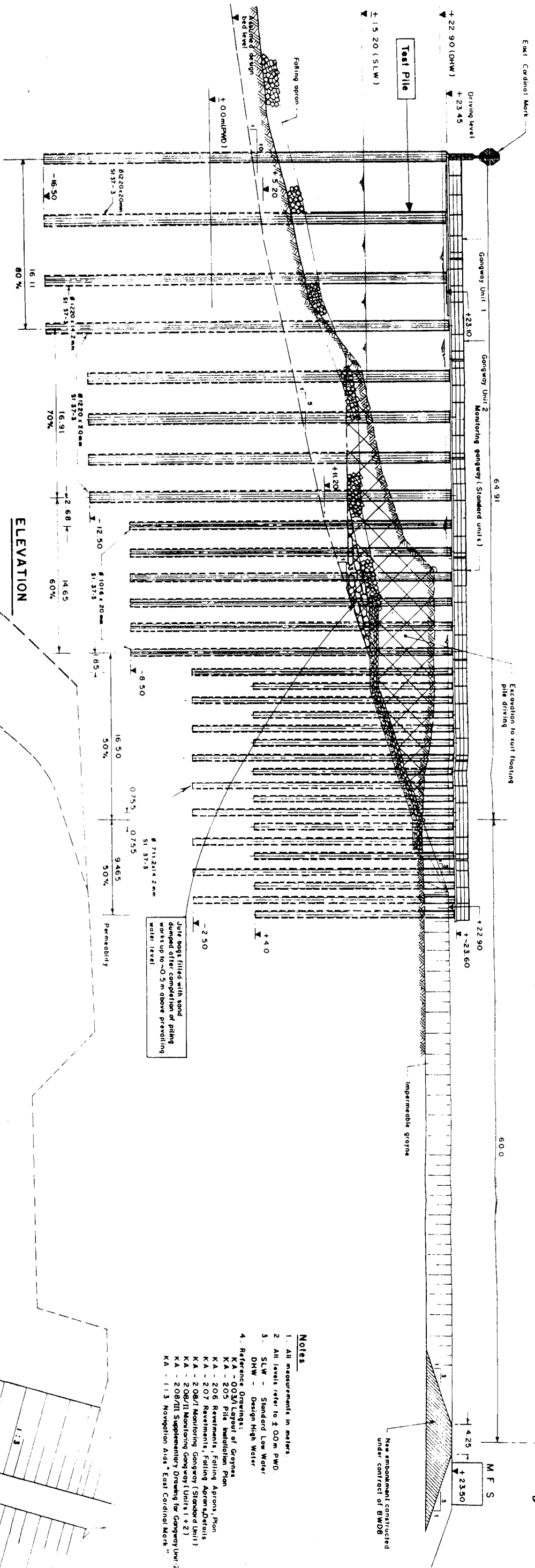
GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH
MINISTRY OF IRRIGATION WATER DEVELOPMENT & FLOOD CONTROL
FLOOD PLAN COORDINATION ORGANIZATION (FPCO)

as per actual site conditions and prevailing water level

4. Reference Drawings:

- KA - 003 Layout of Gaynes
- KA - 005 Filter Gradation Curves
- KA - 006 Stone Gradations and Layer Thicknesses.
- KA - 308 Revetment and Filling Aprons, Detailed Sections.
- KA - 010 General Installation Method, Geotextile Filter Materials

4233317



Falling apron
Concrete blocks 30 cm
(≥ 35 blocks per 1 m² area of falling apron)

Falling apron
Concrete blocks 30 cm
(≥ 35 blocks per 1 m² area of falling apron)

Original bank line

25m behind averaged bank line

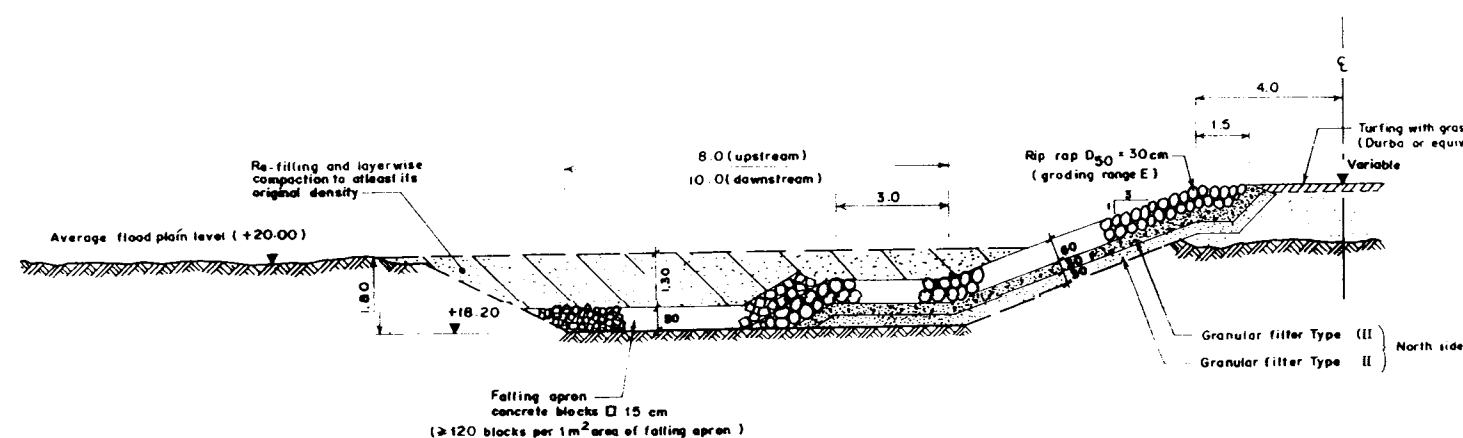
Rip-rap D₅₀ = 30 cm
(grading range E)

Granular filter Type III
Granular filter Type II

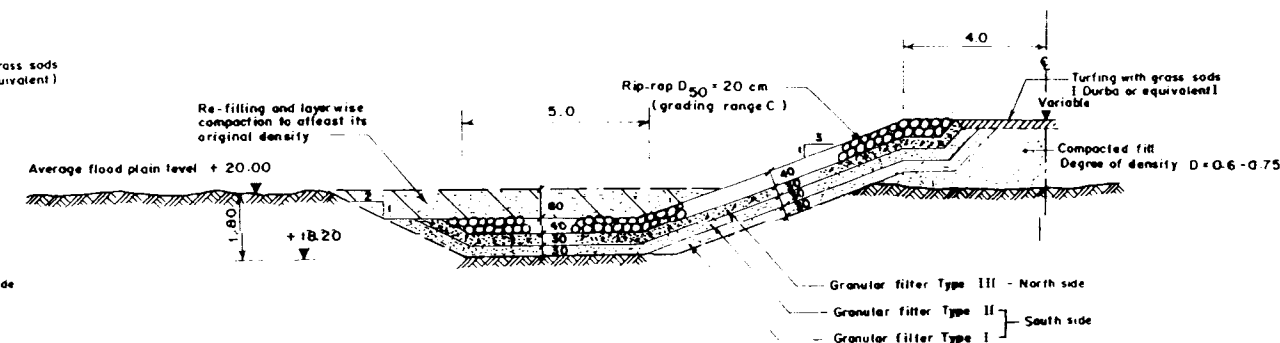
Turfing with grass sods
(Durba or equivalent)

SECTION A-A

Attention: Sections B-B and C-C
North-side of groyne
(as depicted on the drawing): Granular filters II + III
South-side of groyne: Granular filters Type I + II + III
Total thickness of complete filter
≥ 90 cm



SECTION B-B



SECTION C-C

Notes:

- Measurements are in meter and centimeter.
- Levels refer to ±0.0 m PWD.
- Reference Drawings:
KA - 206 Revetments, Falling Aprons, Plan
KA - 005 Filter Gradation Curves.
KA - 006 Stone Gradation and Layer Thicknesses.

DRAWING PHOTOREduced BY 50 %

REV	DATE	NAME	DESCRIPTION	APPROVED
3	26-95	ANOWAR	AS BUILT DRAWING	ag-2005
2	13-12	ANOWAR	LEVELS, MEASUREMENTS	L
1	26-9	ANOWAR	SECTION A-A, BRICK MATTRESS	20

GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH
MINISTRY OF IRRIGATION WATER DEVELOPMENT & FLOOD CONTROL
FLOOD PLAN COORDINATION ORGANIZATION (FPCO)

BANK PROTECTION PILOT PROJECT FAP-21



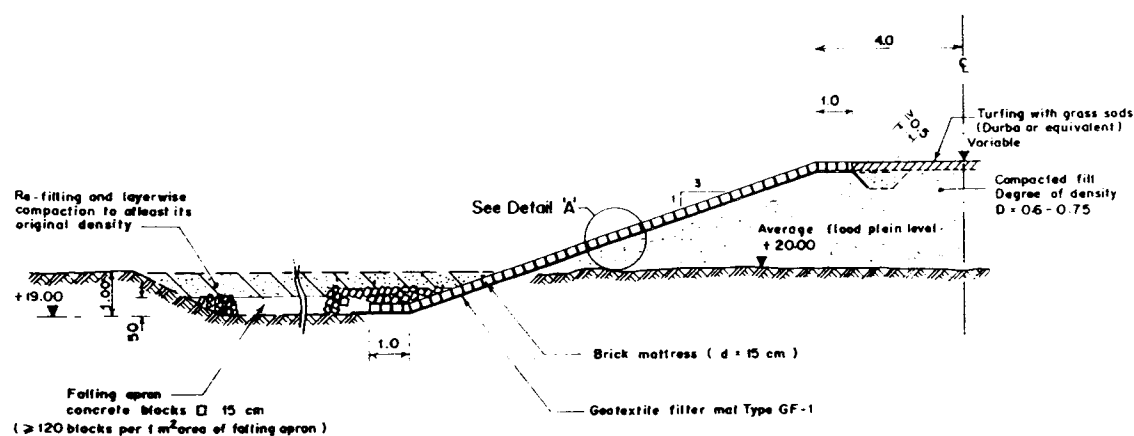
CONSULTING CONSORTIUM

MEMBER: BGS, GBS, MBL, BORTHOMBERG, to association with:
GEOTECHNICAL ENGINEERING & CONSTRUCTION, BANGALORE
GEOTECHNICAL ENGINEERING & CONSTRUCTION, BANGALORE
GEOTECHNICAL ENGINEERING & CONSTRUCTION, BANGALORE

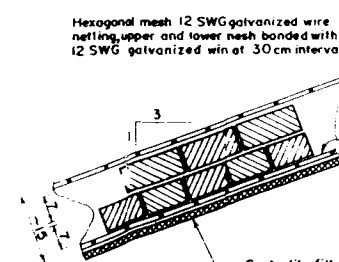
TEST SITE I - KAMARJANI

GROYNE G-2
REVTMENTS FALLING APRONS
DETAILED SECTIONS

NAME	DATE	SCALE	REVISION
ANOWAR	07-12-93	1:100	
ANOWAR	24-01-94		
ANOWAR	10-02-94		

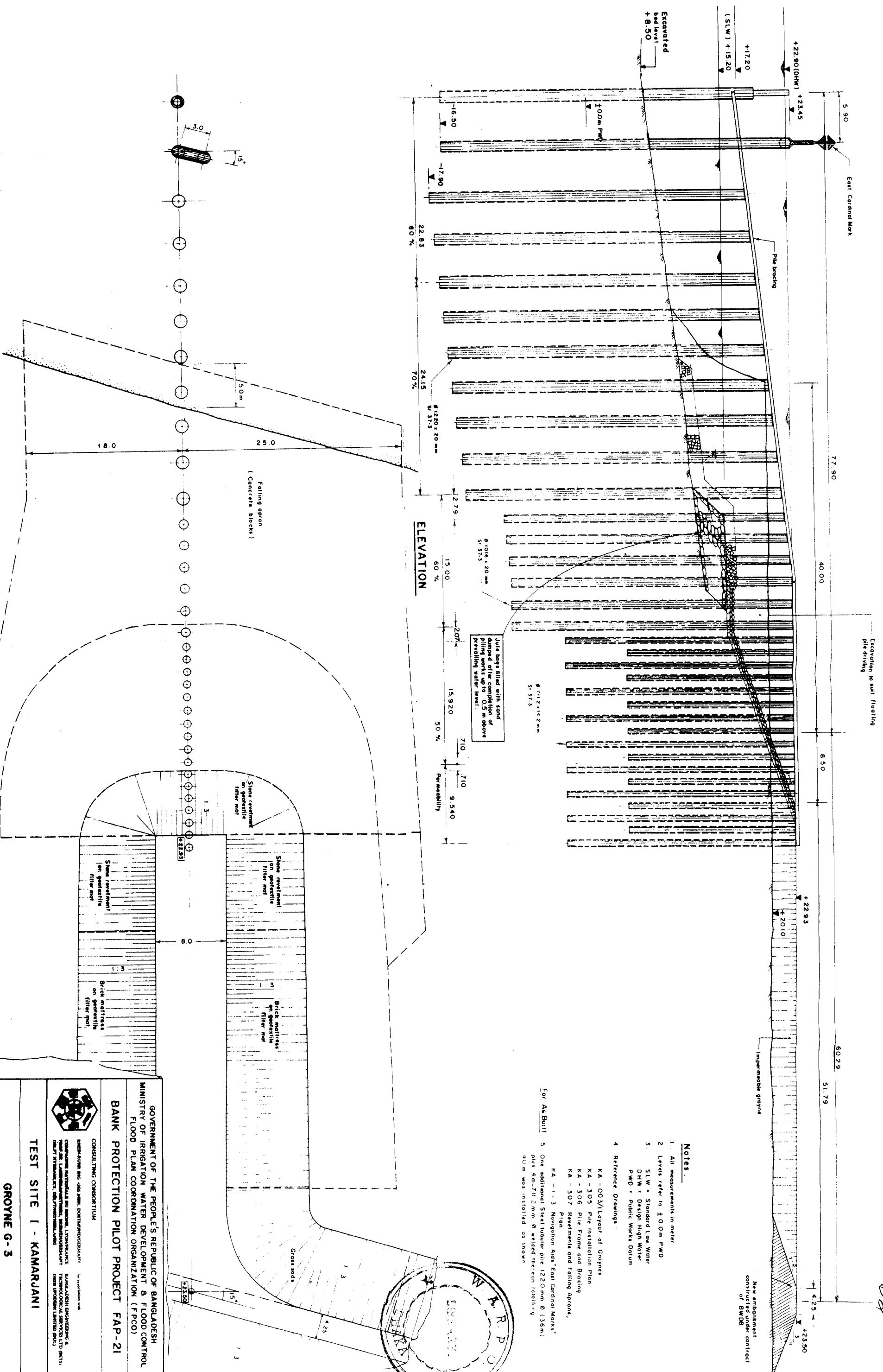


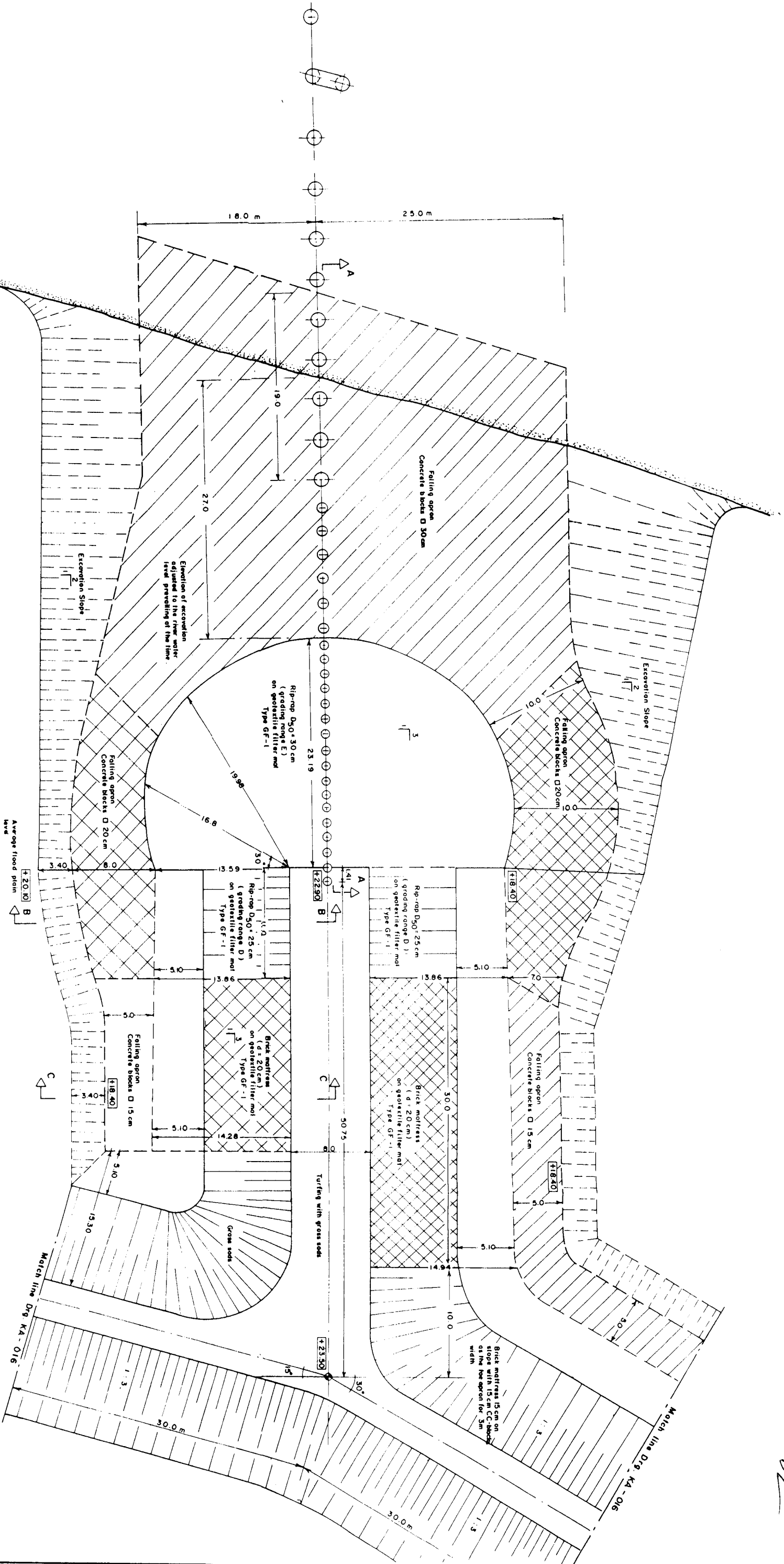
SECTION D-D



SECTION A-A

DETAIL 'A'
BRICK MATTRESS (15 cm)





PLAN

- Notes:
- 1 All measurements in meter
 - 2 Levels refer to ± 0.0 m PWD
 - 3 Reference Drawing
KA - 003/Layout of Groynes
KA - 308 Revetments and falling aprons;
Detailed Sections

DRAWING PHOTO REDUCED BY 50 %

REV.	DATE	NAME	DESCRIPTION	APPROVED
3	16-05	Asst. Engineer (New Sheet)		
2	12-12	Asst. Engineer	Sheet Line, Levels	
1	28-8	Asst. Engineer	Pre Layout	

GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH
MINISTRY OF IRRIGATION WATER DEVELOPMENT & FLOOD CONTROL
FLOOD PLAN COORDINATION ORGANIZATION (FPCO)

BANK PROTECTION PILOT PROJECT FAP-21

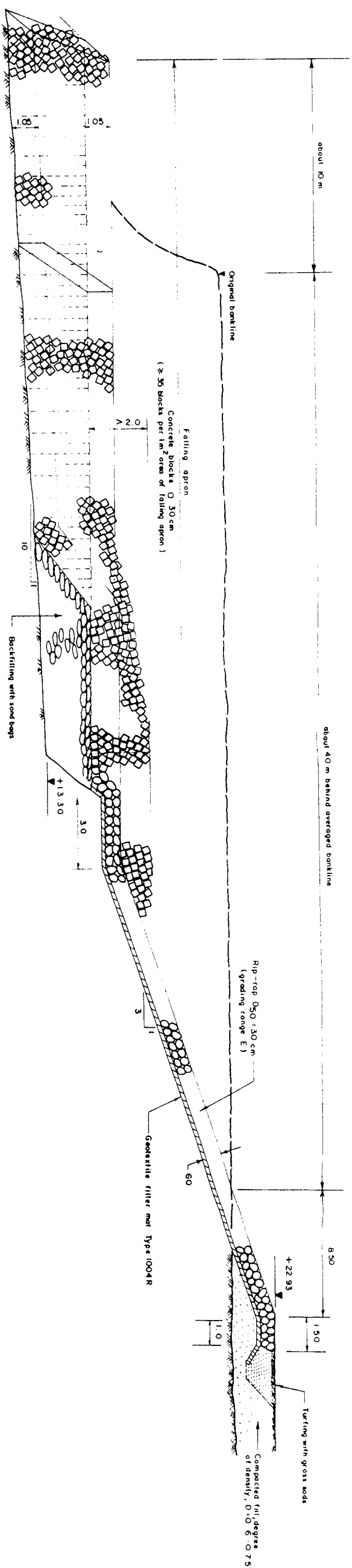
CONSULTING CORPORATION

TEST SITE I - KAMARJANI

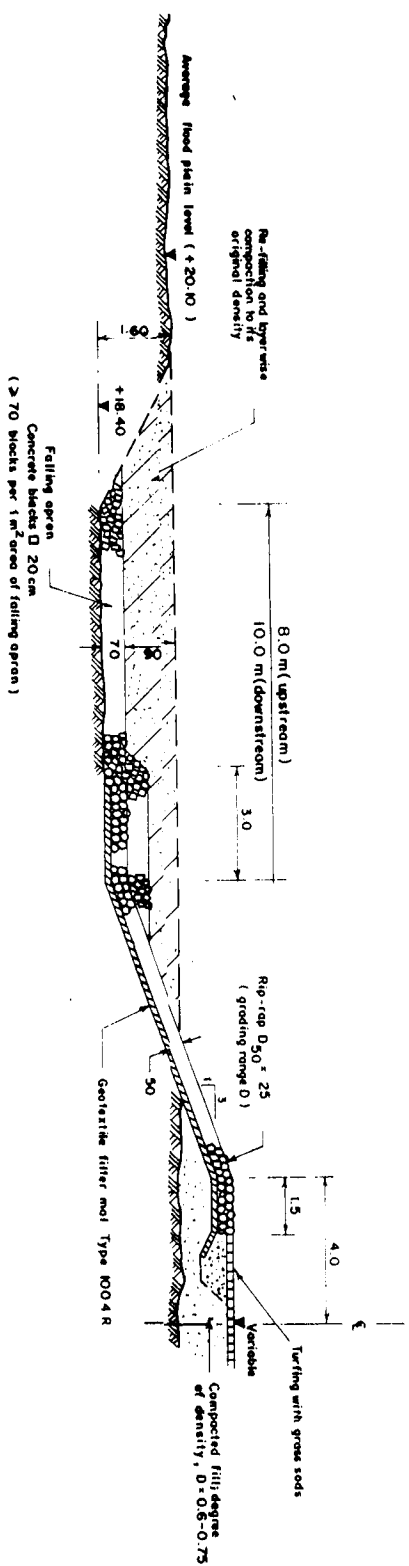
**GROYNE G-3
REVELEMENTS AND FALLING APRONS
PLAN**

NAME	DATE	SCALE
AMWAR	06-12-93	1:200

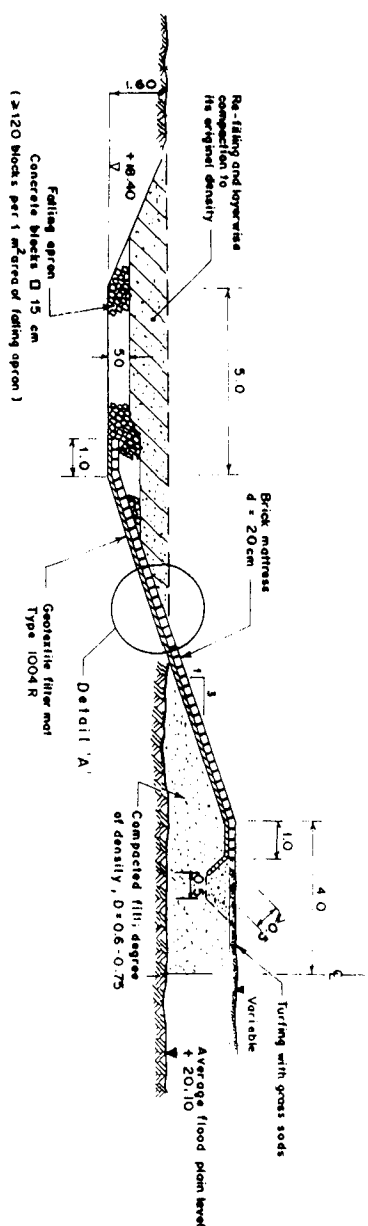
DRAWING NO.	REVISION
KA-307	3



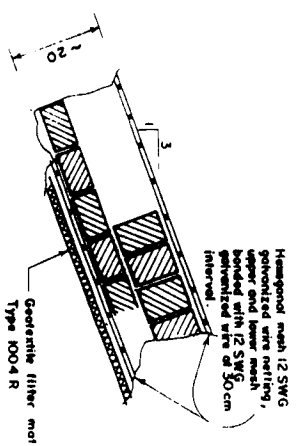
SECTION A - A



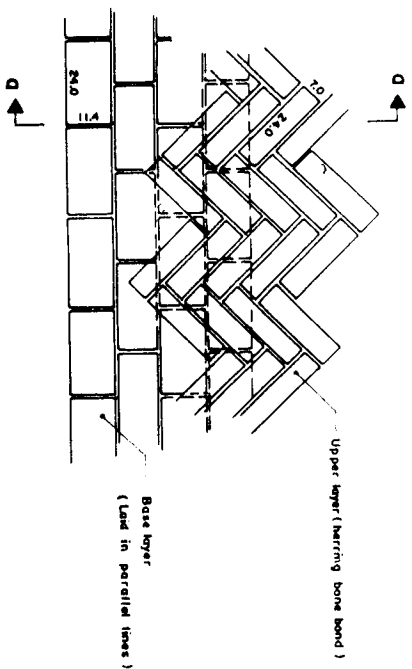
SECTION B-B



SECTION C-C

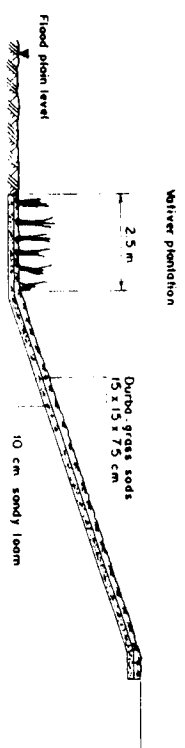


SECTION D-D



DETAIL 'A'
BRICK MATTRESS (d = 20 cm)

SCALE . 1 : 10



TYPICAL SECTION WITH BIOTECHNICAL REVETMENT
SCALE 1 : 100

SCALE 1 : 100

- Notes:**
1. All measurements in meter and centimeter
 2. Levels refer to ± 0.0 m PWD
 3. Reference Drawings:
 - KA-005 Filter Gradation Curves
 - KA-006 Stone Gradation and Layer Thickness
 - KA-307 Revolutions and Falling Apron, "Plan"
 4. All the geosynthetic sheets are pegged (bottom & top)

REV.	DATE	NAME	DESCRIPTION	APPROVED	BY	DATE	DRAWING NO.	REVISION
3	7-4-93	FROESAN	AS BUILT DRAWING	40-10	BRAM	07-12-93	1: 10, 1: 100	
2	8-8-90	AMORAN	LEVELS, REASUREMENTS	6	CHECKED	25-01-94		
REV.	DATE	NAME	DESCRIPTION	APPROVED	BY	DATE	DRAWING NO.	REVISION
						10-02-94	KA-308	3

GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH
 MINISTRY OF IRRIGATION WATER DEVELOPMENT AND FLOOD CONTROL
 FLOOD PLAN COORDINATION ORGANIZATION (FPCO)

BANK PROTECTION PILOT PROJECT FAP-21

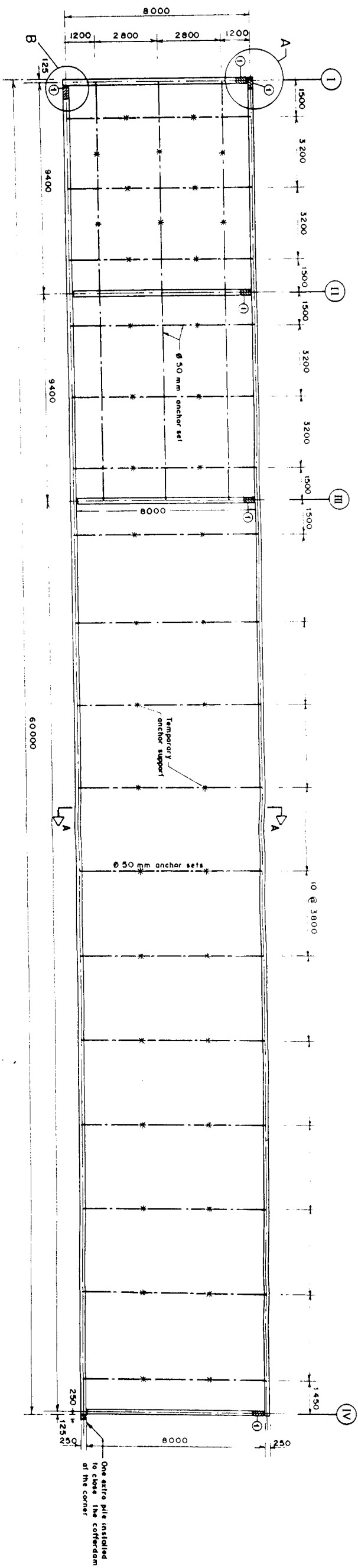
CONSULTING CONSTRUCTION

MANAGING THE JOB: JAMAL BOUTHELOUPOULOS
 MANAGING THE PROJECT: JAMAL BOUTHELOUPOULOS
 PROJECT MANAGER: JAMAL BOUTHELOUPOULOS
 PROJECT MANAGER: JAMAL BOUTHELOUPOULOS

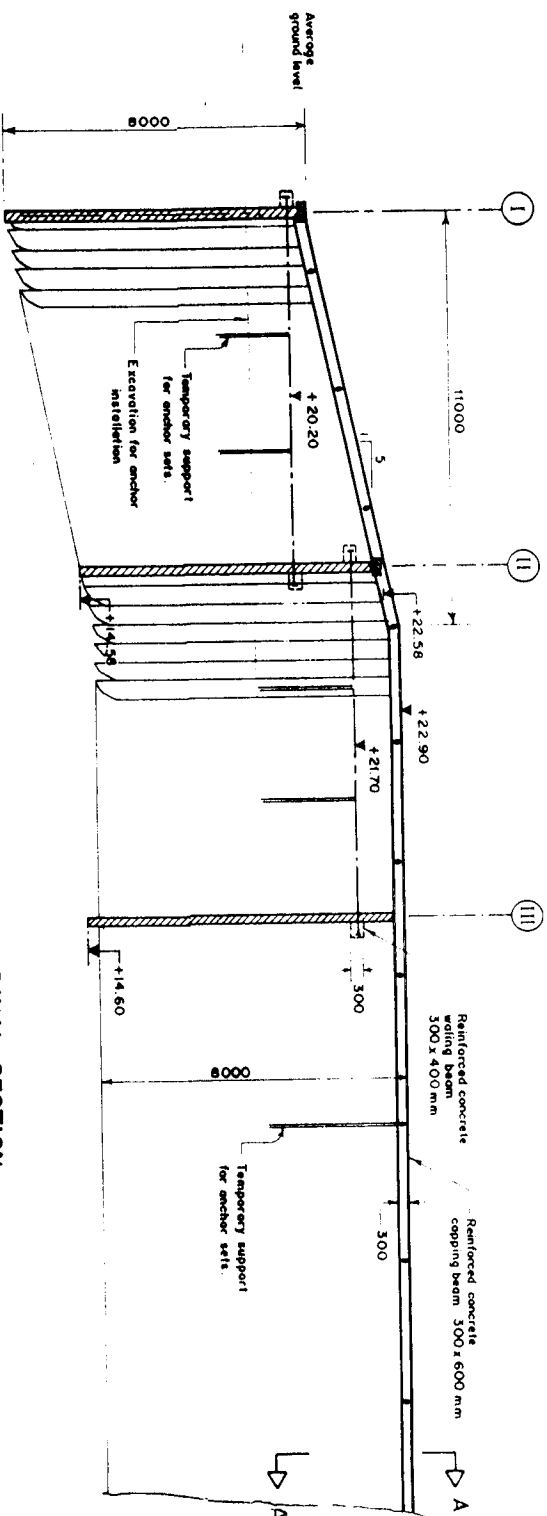
TEST SITE 1 - KAMARJANI

GROYNE G-3
 REVEMENTS AND FALLING APRONS
 DETAILED SECTIONS

BANGLADESH GOVERNMENT &
 TECHNICAL SERVICES LTD (BTS)
 ONE, UPOKSHI LAMTOL, DHAUL

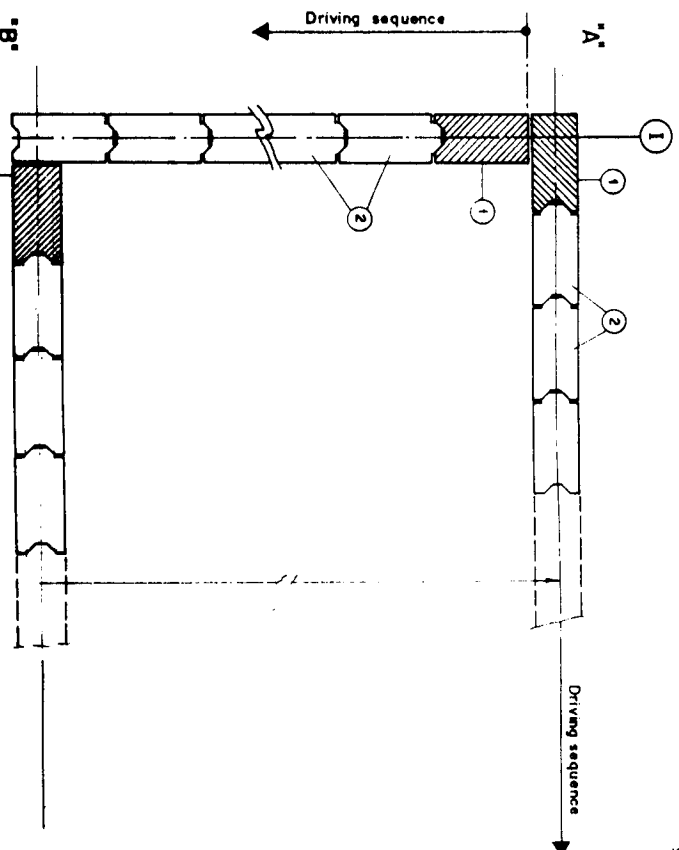
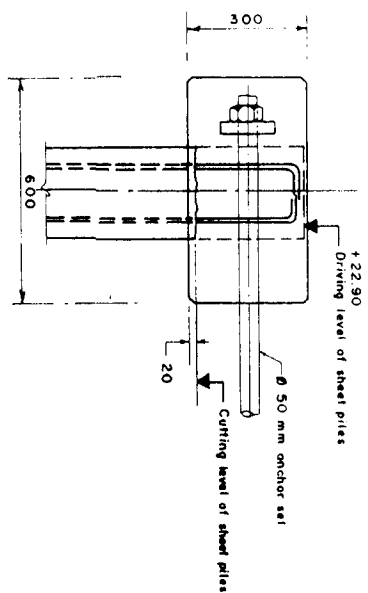


GENERAL LAYOUT PLAN
Scale 1:100

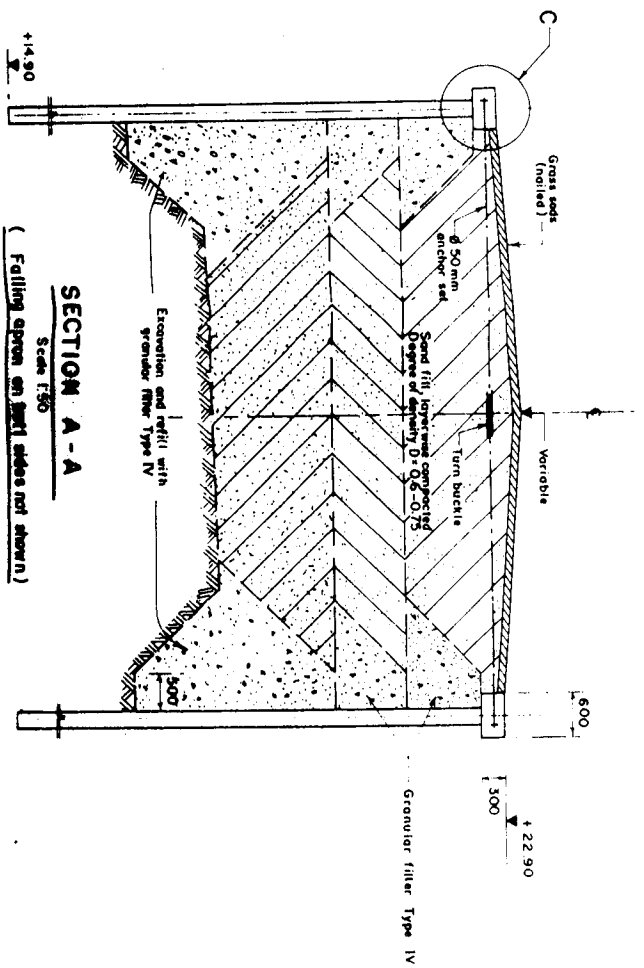


PART LONGITUDINAL SECTION
Scale 1:100

DETAIL - C
Scale 1:10



DETAIL - A-B-B
Scale 1:20



SECTION A-A
Scale 1:50

REV.	DATE	NAME	DESCRIPTION
2	2-8-84	Author	As built drawing
1	8-11-84	Author	Section A-A, Actual ground level

REV.	DATE	NAME	DESCRIPTION
2	2-8-84	Author	As built drawing
1	8-11-84	Author	Section A-A, Actual ground level

GROYNE G - A
CONCRETE SHEET PILE COFFERDAM
PILE INSTALLATION PLAN DETAILS

TEST SITE 1 - KAMARJANI

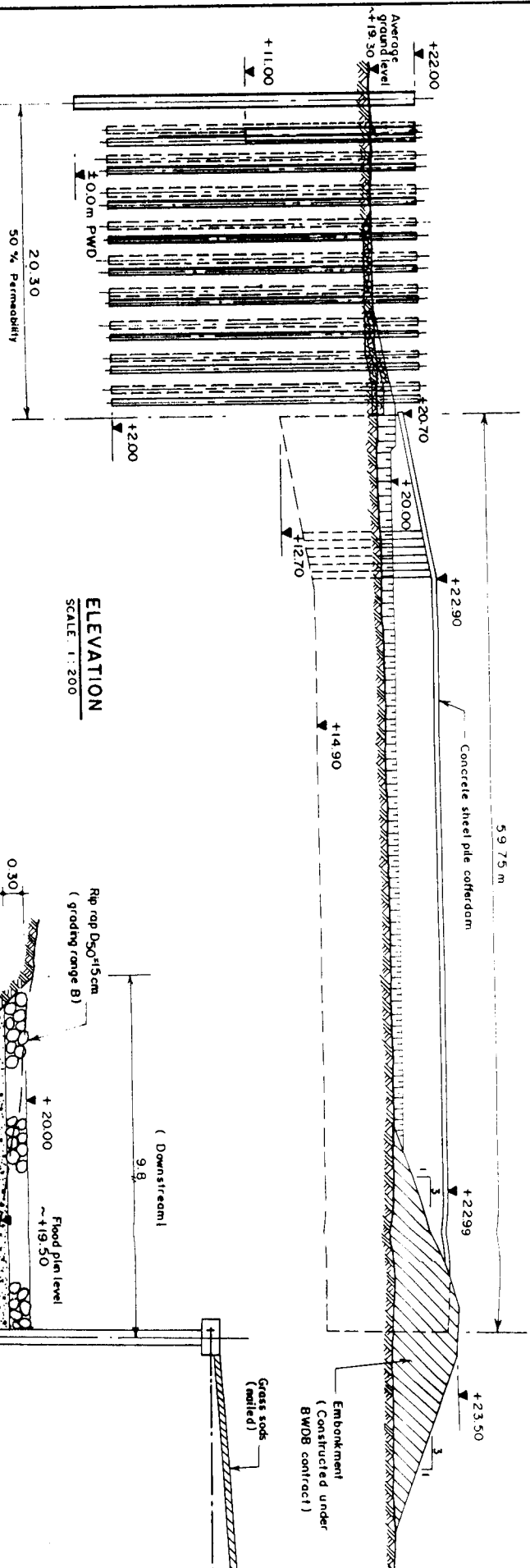
CONSULTING CONSULTANT
ENGINEERING & ARCHITECTURE
BY ASSOCIATION WITH
BANGALADESH ENGINEERING & ARCHITECTURE
PRACTICE LIMITED (BEAPL)
100, RAJSHAHI ROAD, RAJSHAHI, DHAKA
1000, BANGLADESH

BANK PROTECTION PILOT PROJECT FAP-21

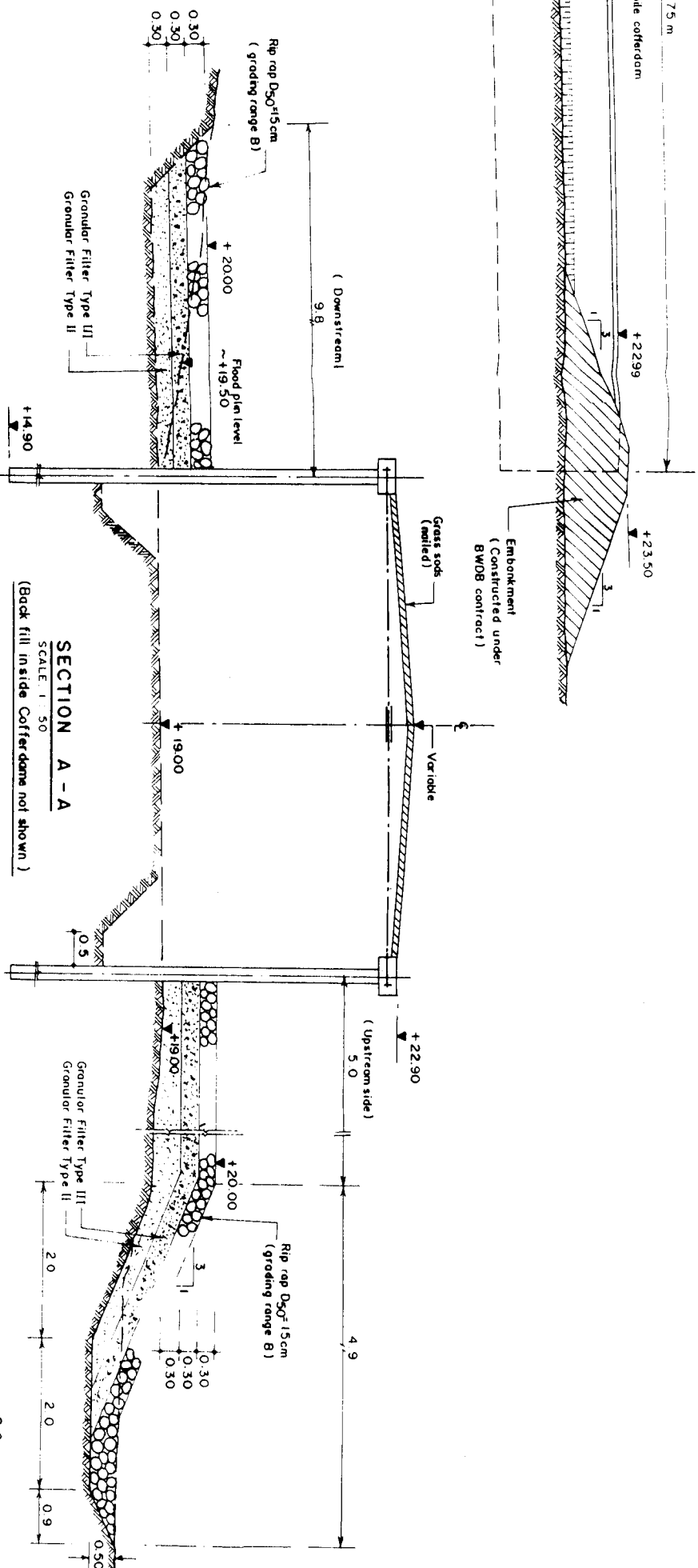
GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH
MINISTRY OF IRRIGATION WATER DEVELOPMENT & FLOOD CONTROL
FLOOD PLAN COORDINATION ORGANIZATION (FPCO)

DRAWING PHOTO REDUCED BY 50 %

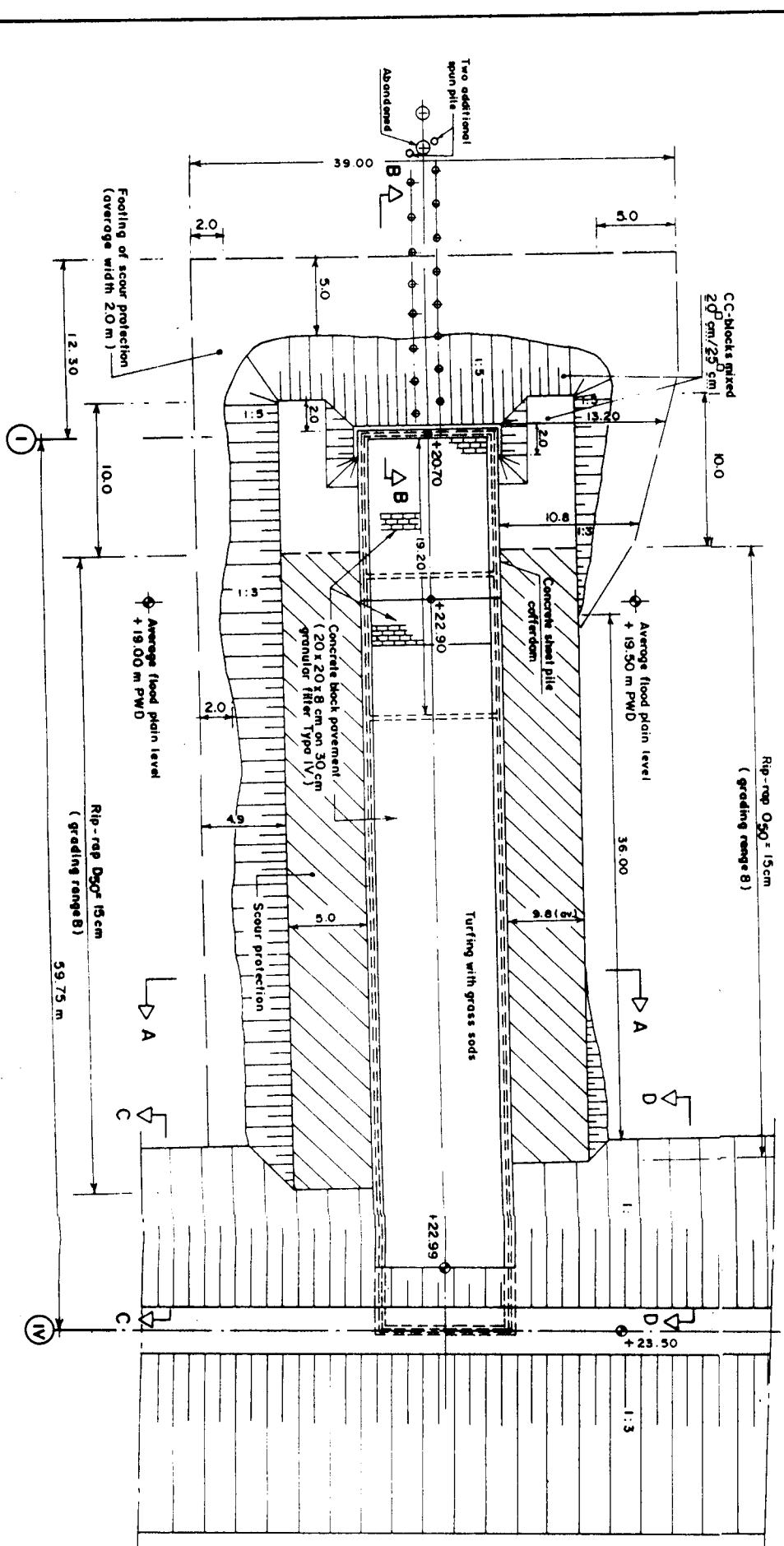
- Notes:
- All measurements in millimeters.
 - Levels refer to 100.0m PWD.
 - Sheet pile coping beam and waling beams:
 - Reinforcing steel grade 60, ASTM A-615
 - Concrete class B25, DIN 1045
 - Cement Type I (Ruby brand, Bangladesh)
 - Concrete cover to reinforcing steel 30 mm
 - ① - Starter pile
 - ② - Standard pile
 - Reference Drawings:
 - KA - 003 Layout of Groyne
 - KA - 403 Reinforced Concrete Sheet Piling
 - KA - 407 Details of Anchoring
 - KA - 408 Coping Beams, Waling Beams
 - KA - 414 Details of Scour Protection



ELEVATION
SCALE 1:200



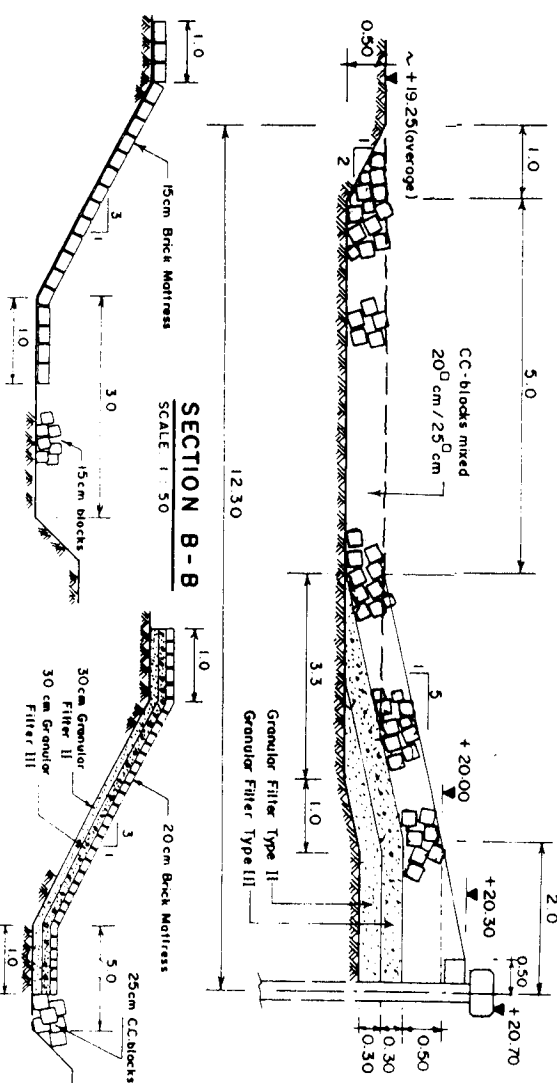
SECTION A - A
SCALE 1:50
(Back fill inside Cofferdam not shown)



PLAN
SCALE 1:200

- NOTES:**
1. In case Rip-rap, grading range B, is not available in adequate quantity, CC-blocks of mixed size 150 cm and 200 cm can be used instead.
 2. All measurements in meter.
 3. All levels refer to ± 0.00 PWD.
 4. Length of slopes adopted to existing ground levels around the cofferdam.
 5. Reference Drawings: KA-401 Groyne G-A General Arrangement Plan and Elevation.

SECTION C - C
SCALE 1:50



SECTION B - B
SCALE 1:50

SECTION D - D
NOT TO SCALE

DRAWING PHOTOREDUCTION BY 50 %

REV.	DATE	NAME	DESCRIPTION	APPROVED	DATE	NAME	DESCRIPTION	APPROVED
1	3-5-95	Amrinder	As Built Drawing	As Built	17-12-94	Amrinder	As Built Drawing	As Built
2	18-12-94	Amrinder	As Built Drawing	As Built	18-12-94	Amrinder	As Built Drawing	As Built

**CONCRETE SHEET PILE COFFERDAM
DETAILS OF SCOUR PROTECTION**

TEST SITE 1 - KAMARJANI

**GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH
MINISTRY OF IRRIGATION, WATER DEVELOPMENT & FLOOD CONTROL
FLOOD PLAN COORDINATION ORGANIZATION (FPCCO)**

BANK PROTECTION PILOT PROJECT FAP-21

CONSULTING CONTRACTOR

CHANGING ENGINEERING & DESIGN, LITERATURE, TECHNICAL SERVICES LTD. (C.E.D.L.)

CHANGING ENGINEERING & DESIGN, LITERATURE, TECHNICAL SERVICES LTD. (C.E.D.L.)

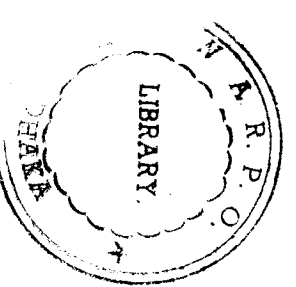
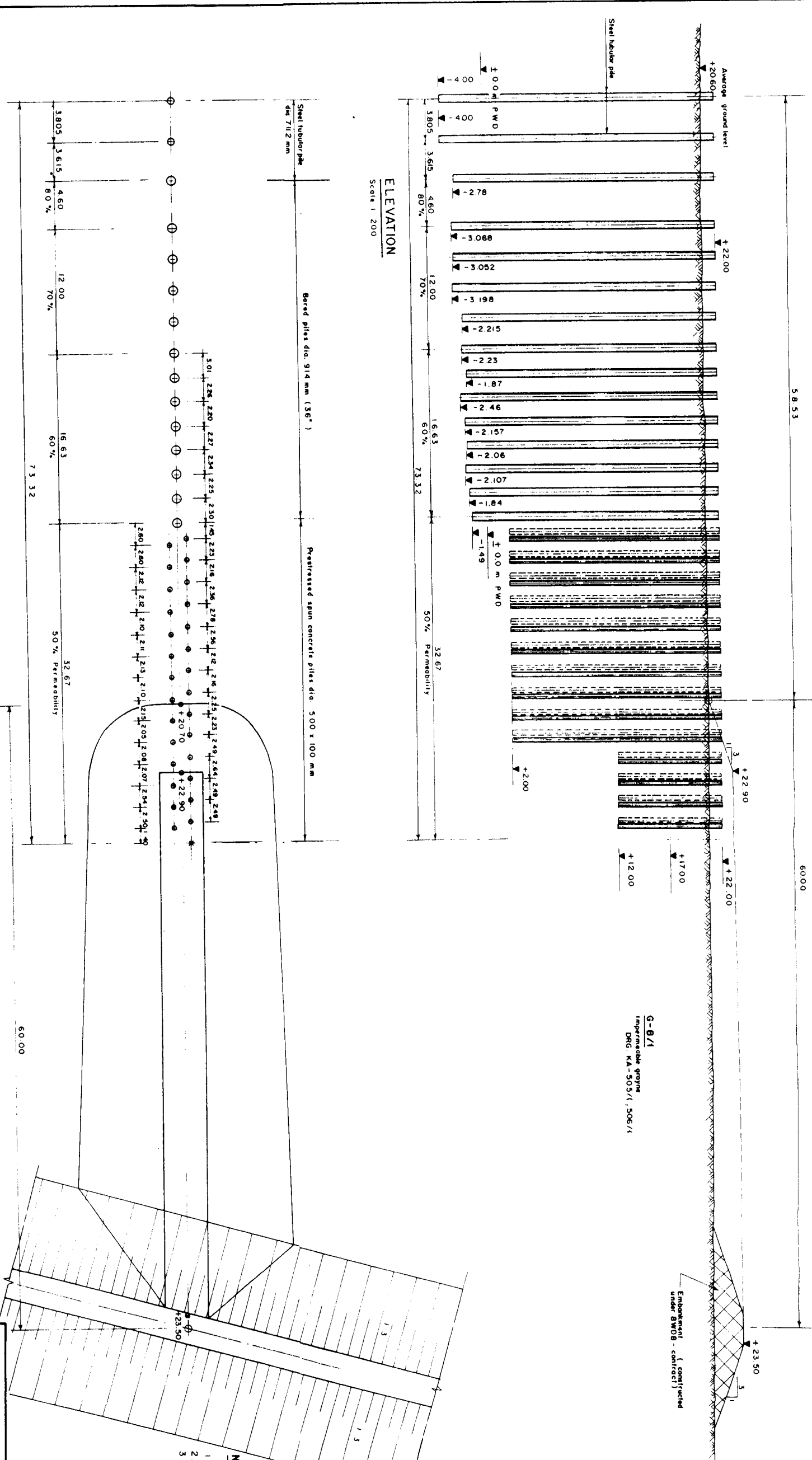
CHANGING ENGINEERING & DESIGN, LITERATURE, TECHNICAL SERVICES LTD. (C.E.D.L.)



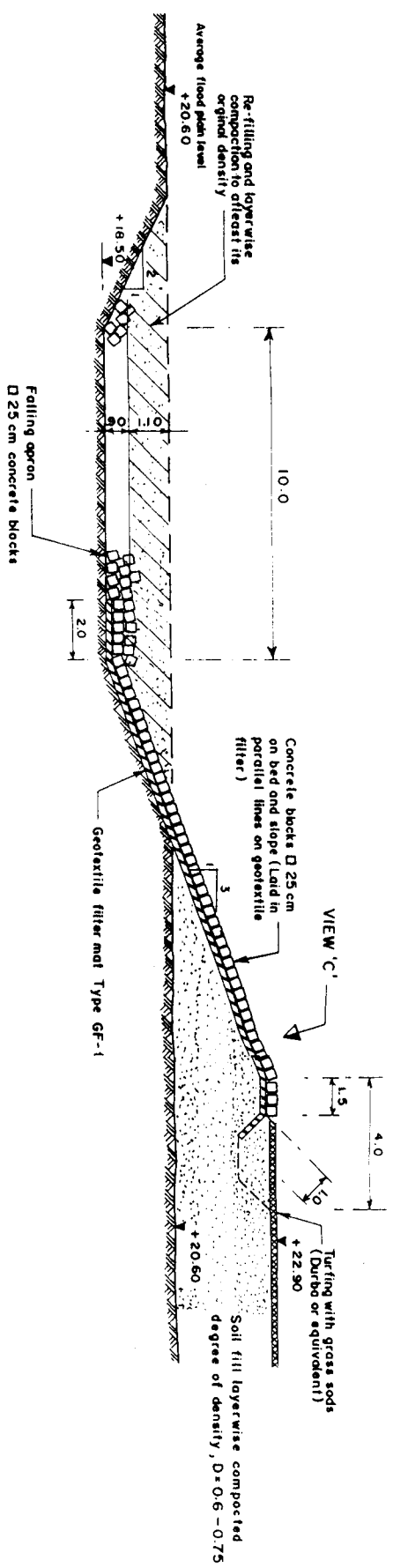
CHANGING ENGINEERING & DESIGN, LITERATURE, TECHNICAL SERVICES LTD. (C.E.D.L.)

CHANGING ENGINEERING & DESIGN, LITERATURE, TECHNICAL SERVICES LTD. (C.E.D.L.)

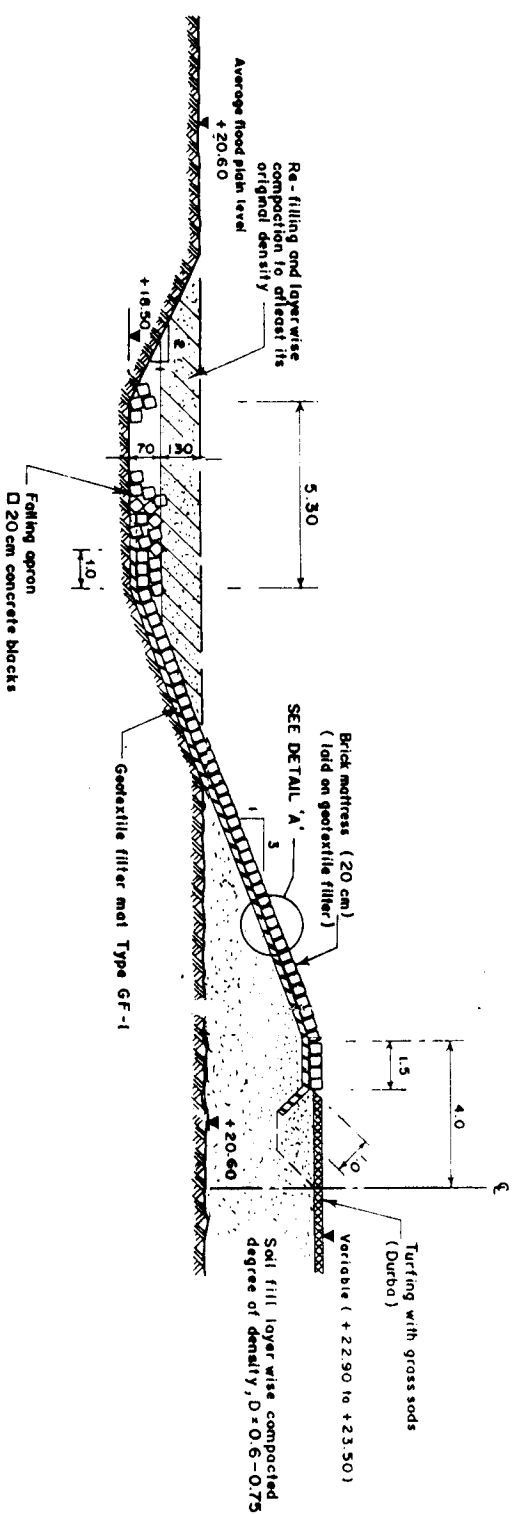
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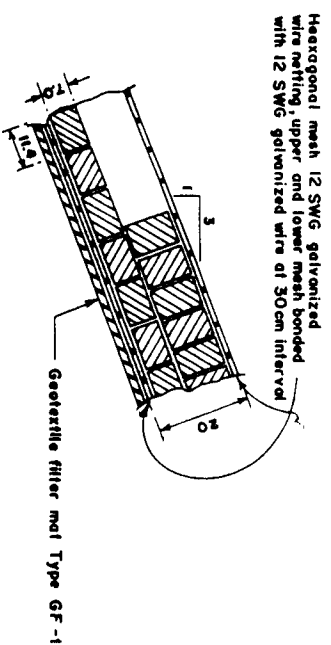
GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH			
MINISTRY OF IRRIGATION WATER DEVELOPMENT & FLOOD CONTROL			
FLOOD PLAN COORDINATION ORGANIZATION (FPCO)			
BANK PROTECTION PILOT PROJECT FAP-21			
CONSULTING CONSULTANT			
TEST SITE 1 - KAMARJANI			
GROYNE G-8/1			
GENERAL ARRANGEMENT			
PLAN, ELEVATION			
DRAWING PHOTO REDUCED BY 50 %			
REV.	DATE	NAME	DESCRIPTION
2	10-05	ANONIM	AS BUILT DRAWING
1	10-12	ANONIM	GROUND LEVELS, PILES AXES & TO 8.16
1	10-02-94	ANONIM	APPROVED
NOTES			
1 All measurements in meter.			
2 All levels refer to ±0.00 PWD			
3 Reference Drawings KA-504/1 Pile Installation Plan			
LIBRARY			



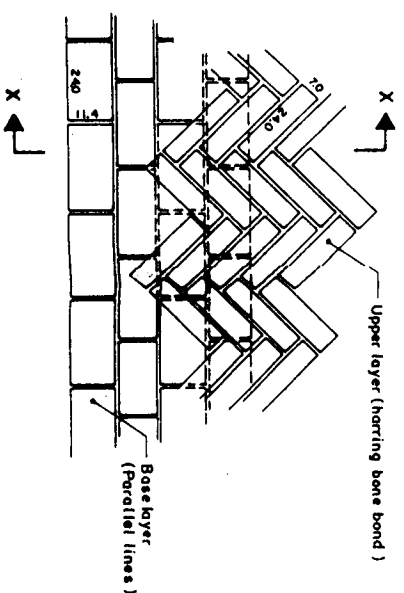
SECTION A-A
SCALE 1:100



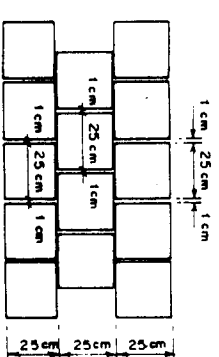
SECTION B-B
SCALE 1:100



SECTION X-X



DETAIL 'A'
BRICK MATTRESS (d = 20 cm)
SCALE 1:20



VIEW 'C'
CONCRETE BLOCKS
SCALE 1:20

NOTES:

- Measurements in meter, unless shown otherwise.
- Levels refer to ± 0.00 m PWD.
- Reference Drawings:

KA - 005 Filter Gradation Curves.
KA - 505/1 Groynes G-8/1
Revetments, Filling Aprons
Plan.

DRAWING PHOTOREDUCTION BY 50 %

REV	DATE	NAME	DESCRIPTION	APPROVED
1	22-9-95	ANWAR	AS BUILT DRAWING	4-1-00

GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH
MINISTRY OF IRRIGATION WATER DEVELOPMENT & FLOOD CONTROL
FLOOD PLAN COORDINATION ORGANIZATION (FPCCO)

BANK PROTECTION PILOT PROJECT FAP-21

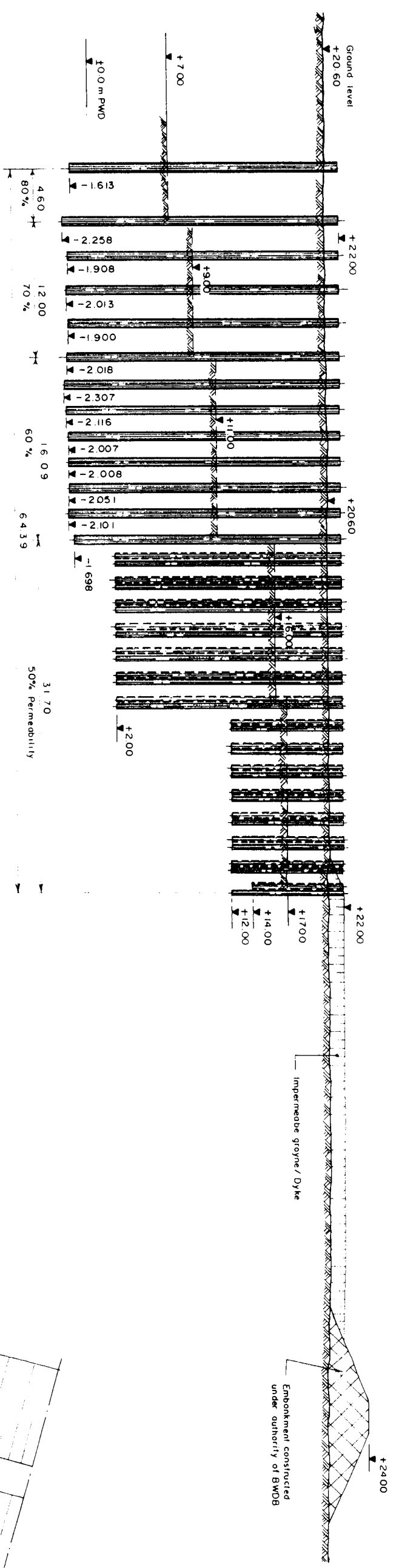
CONSULTING ORGANIZATION

DESIGNED AND DRAWN BY: MR. ANWAR
CHECKED BY: MR. ANWAR
APPROVED BY: MR. ANWAR

TEST SITE I - KAMARJANI

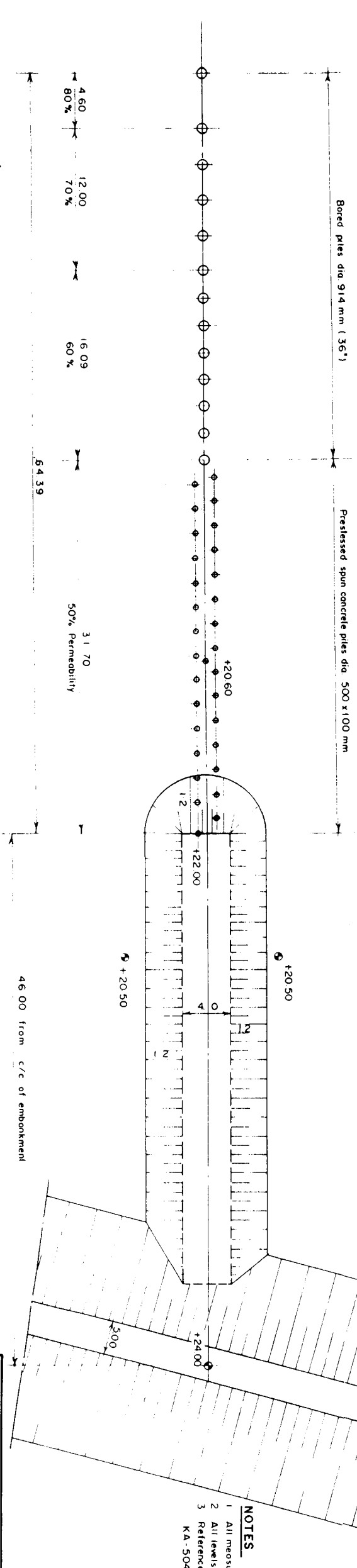
GROYNE G-8/1
DETAIL SECTIONS

NAME	DATE	SCALE
ANWAR	06-12-94	1:100, 1:20, 1:10
DESIGNED	CHECKED	APPROVED
ANWAR	13-12-94	ANWAR
DRAWING NO.	KA-508/1	REVISION
		1



ELEVATION

Scale 1:200



PLAN

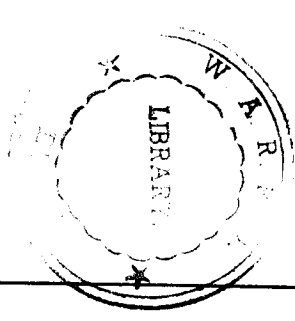
Scale 1:200

- NOTES
1. All measurements in meter
 2. All levels refer to ±0.0 m PWD
 3. Reference Drawings KA-504/2 Pile Installation Plan

DRAWING PHOTOREDUCED BY 50 %

REV.	DATE	NAME	DESCRIPTION
1	06.09.95	ANWAR	AS BUILT DRAWING (NEW SHEET)

GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH MINISTRY OF IRRIGATION, WATER DEVELOPMENT & FLOOD CONTROL FLOOD PLAN COORDINATION ORGANIZATION (FPCO)			
BANK PROTECTION PILOT PROJECT FAP-21			
CONSULTING CONSULTANT			
BANGLADESH ENGINEERING & ARCHITECTURE CONSULTANTS ASSOCIATION (BEACA)			
TEST SITE 1 - KAMARJANI			
GROYNE G-B/2			
GENERAL ARRANGEMENT			
PLAN, ELEVATION			
NAME	DATE	SCALE	1:200
ANWAR	06-12-93		
CHECKED	26-01-94	DRAWING NO.	KA-501/2
APPROVED	10-02-94	REVISION	1



ANNEX B

(Reserved for Final Report)

ANNEX C

(Reserved for Final Report)

ANNEX D

(Reserved for Final Report)

ANNEX E

Model Tests

- Photographs



Photo 1: Return flow reduced by adaptation of pile spacing and impermeable head; T1, flow from G-2 to G-3

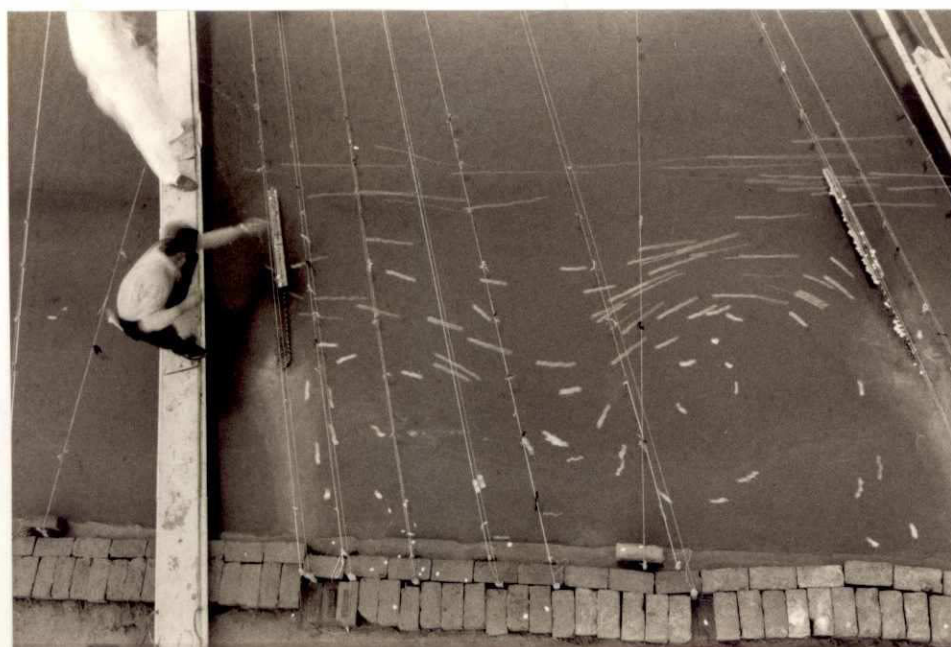


Photo 2: Return flow becomes unstable pattern of two eddies after retreating the bank. No further reduction of bank flow velocities; T2, flow from G-2 to G-3

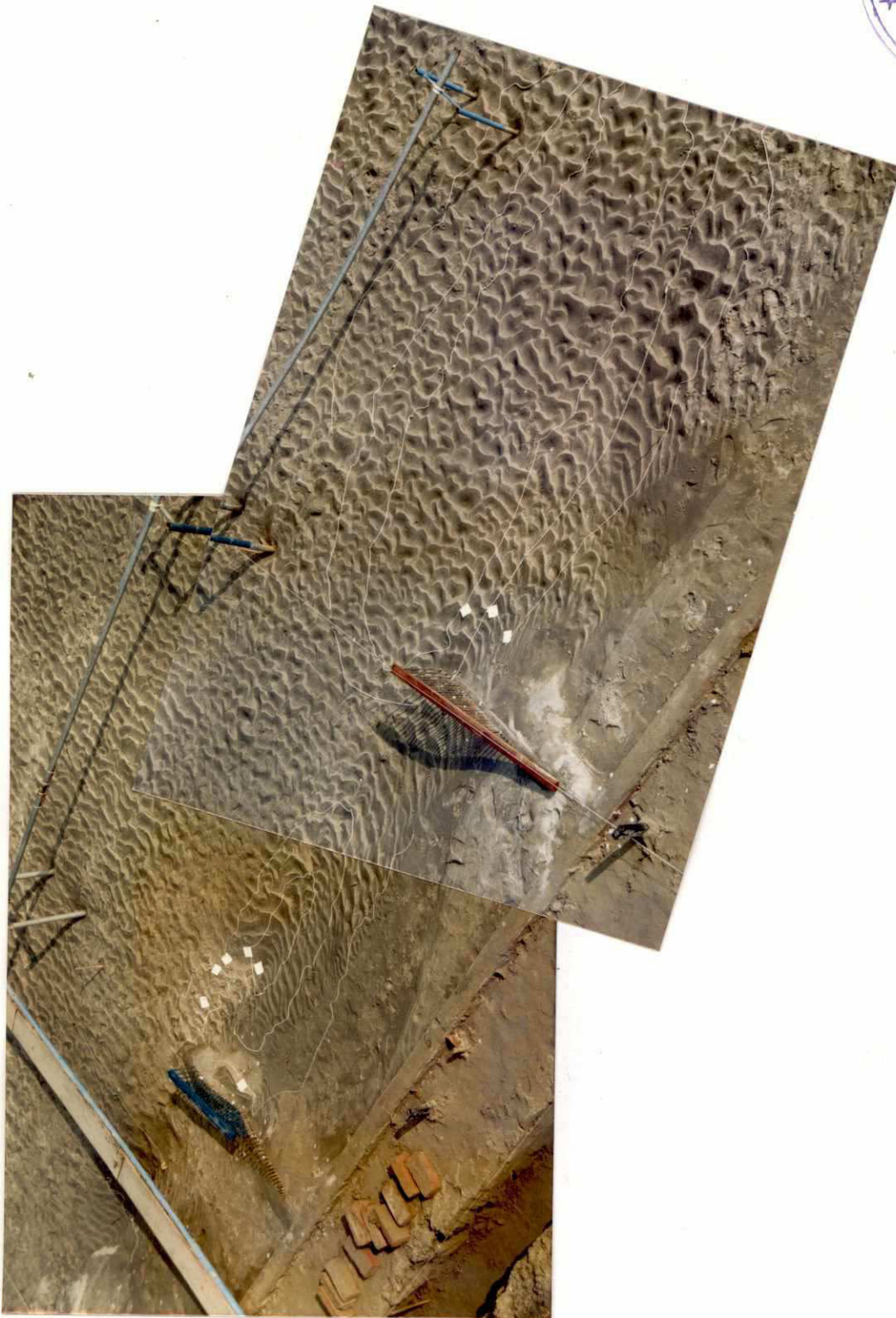
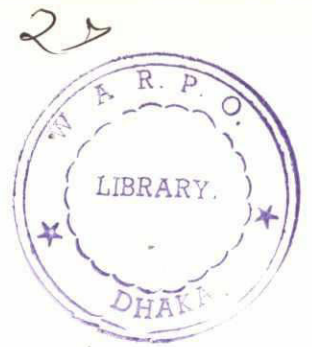


Photo 3 and 4: Local scour holes downstream from G-2 and G-3 in T2

ANNEX F

**Detailed Description
of Works**

DETAILED DESCRIPTION OF WORKS

1 INITIAL MEASURES FOR ADAPTATION OF PILE STRUCTURES AT GROYNES G-A AND G-3

Utilizing steel pile material available at Site the following works shall be carried out:

Groyne G-A (Drawing No. AD-KA-040/-041)

- Welding at the pile yard of three piles dia. 1016 x 20 mm, lengths 32.0 m Nos.) and 31.5 m (pile axes 25-26-27);
- Extension of groyne piles at axes 22-23-24 by 4.5 m, after cutting about 10 cm from the present pile heads;
- Welding at the pile yard of four piles dia. 711.2 x 14.2 mm, length 32 m each (pile axes 18-19-20-21);
- Installation of piles No. 25-26-27 as well as No. 18-19-20-21, re-driving of piles No. 22-23-24. Appropriate pile installation records are to be maintained as per Specifications, Subsection 1560.
- Cutting of all steel piles to the level shown on the drawing, subsequent welding of steel cover plates and horizontal steel beam bracings;
- Pile No. 27 shall be extended after installation by an about 7 m long steel pile section dia. 711.2 x 14.2 mm and finally be topped with a "Danger Point Mark", whose conical shaped top is to be painted "RED", using durable, preferably epoxy-resin based paint, after proper derusting and primer coating.
- In a 2nd Phase, i.e. after arrival at Site of the newly imported steel pile material, 5 Nos. additional piles dia. 711.2 x 14.2 mm, length 32 m each, shall be prepared and installed at locations No. 17-16-15-14-13 as supplementary piles.

Groyne G-3 (Drawing No. AD-KA-031)

- Preparation of 8 Nos. pile extension pieces dia. 711.2 x 14.2 mm, individual lengths of 12 m to 13.5 m, utilizing selected material available at Site;
- Cutting the horizontal pile bracing of the groyne structure between piles No. 2 and 18; material to be stored for re-use;
- Cutting the pile top plates from piles No. 3-5-7-9-11-13-15 and 17; material to be stored for re-use;
- Piles No. 3-5-7-9-11-13-15-17 to be bevelled for welding, pile extension sections to be placed as per drawing, well aligned, secured in place and connected to the pile finally by full penetration butt welding;
- Re-driving of the extended piles to approximately top level +23.00 m PWD. Appropriate pile installation records are to be kept as per Specifications, Subsection 1560;
- Cutting of all redriven piles to finishing level, subsequent welding of steel cover plates and horizontal bracings, utilizing the interim stored materials.
- In a 2nd Phase, i.e. after arrival of the newly imported steel pile material, the groyne will be further extended and modified as detailed under Chapter 4.

2 RELOCATION AND IMPROVEMENT OF MAIN EMBANKMENT (Drawing Nos. AD-KA-010/-020/-030/-002/-003)

- (1) Between groynes G-3 and G-2 the present embankment shall be relocated by 50 m towards Hinterland and between groynes G-2 and G-1 by 20 m.

Transitions from the existent embankment terminate 150 m downstream the G-3 and 30 m downstream G-1.

In the area of groynes G-3 and G-2 a gap of about 30 m width each shall be kept open, to facilitate pile installation works within the future groyne head area.

Earth works and subsequent revetment works shall commence 150 m downstream from groyne G-3 and shall progress section-wise towards upstream, following the directions of para (3).

As soon as the bored concrete piles No. 1 to 6 for the new groyne G-A/2 have been completed, the improvement of the existent embankment within ± 50 m up- and downstream from the new groyne shall be taken up. The toe level is to be extended at the river-side to +17.50 m PWD and the trench for the toe protection as well as the launching apron area are likewise to be excavated to level +17.50 m PWD. The excavated soil shall be used for final profiling of the BWDB-embankment in this area, otherwise transported and stock-piled for use in other working sections.

Provided time permits and with the Consultants approval the improvement of the BWDB-embankment from 50 m downstream from G-A/2 towards Manos River may be taken up (OPTIONAL WORK). The toe level of the existent embankment is likewise to be extended to level +17.50 m PWD.

Simultaneously with progressing embankment works, adequate soil material stock piles shall be arranged as close as possible to the future groyne heads G-3 and G-2. This is important to facilitate construction of these parts in the shortest possible period of time after completion of the respective piling works. This work is on the critical path.

- (3) It is essential for the strategy of work implementation that individual embankment sections are built-up and compacted in layers and finally profiled to receive immediately filter and revetment layers. The length of such working sections shall not exceed about 60 m. The toe level of the river-sided slopes are designed at +17.50 m PWD, except between groynes G-2 and G-1, where the excavation level is fixed at +18.50 m PWD.
- (4) The existent borrow pits within the area of the realigned embankment are to be cleaned within the areas to be backfilled from any undesirable matter and filled in well compacted layers, following the design profiles shown on the Drawings.
- (5) Soil from the present embankment which is to be relocated and all soil from cuttings for the slope extensions along the river-sided slopes shall be re-used for construction of the new embankment. Material shortage are to be covered from borrow pits to be arranged by the Contractor.
- (6) The river-sided slope revetment comprises of 15 cm thick brick mattresses as per Specifications, Subsection 1078 and in accordance with the details shown on the Drawings. The mattresses are to be laid on a geotextile filter mat Type GF-1, which will be supplied by the Consultant to the Contractor free of cost at Site.

Between the groynes G-3 and G-1 the top of mattressing is designed at level +21.0 m PWD, which is 30 cm above the Standard High Water level. Above, the earthen slope shall be covered

with Durba-grass sods in accordance with Specifications, Subsection 1079. Due to delivery constraints Geo-Jute soil saver shall not be provided, but the individual grass sods must be properly nailed with bamboo sticks and thoroughly watered daily.

Downstream of groyne G-3, up to and beyond the new groyne G-A/2, the top of brick mattressing is designed at +23.50 m PWD, which is the design top level of the embankment.

It is to be noted that a toe protection along the main embankment by cc-blocks will not be provided, although shown in the Drawings. The reason being to limit constraints in timely production of these blocks, but mainly due to the experience that a toe protection of limited width will not substantially improve the stability of the mattresses in case of severe erosion/scouring along the slopes toe. CC-block toe protection will only be placed within ± 30 m upstream/downstream from the center of groynes G-3 and G-2 and within ± 50 m of groyne G-A/2.

It is further to be noted that (for time constraint reasons) the land-sided slopes of the embankment shall not receive Durba-grass sod coverage in this season, although indicated on the Drawings.

- (7) The toe of the brick mattresses shall be placed against a single row of cc-blocks size 25/25/25 cm. The bottom wire mesh is to be placed around the cc-block row and to be overlapped with the top wire mesh for at least 75 cm. Top and bottom wire mesh are to be tightly bonded with 12 SWG wire at 30 cm interval. Any wire used in brick mattressing shall be galvanized wire.

At the upper end the brick mattressing shall be specially anchored in a trench. The top and bottom wire mesh are to be tight together as anchor flap for at least 60 cm width, using 12 SWG binders. At the end a continuous 12 SWG wire is to be drawn-in (i.e. along the upper end of the wire mesh). Hard wood pegs of size 50 x 50 mm, and at least 60 cm long, shall be driven through the wire mesh at 60 cm interval along the upper end of the anchor flap. The anchor trench is subsequently to be filled with sandy soil in well compacted layers.

3 CONSTRUCTION OF SUPPLEMENTARY GROUYNE G-A/2

(Drawings No. AD-KA-050/-051)

- (1) Severe bankline erosion is expected downstream of groyne G-A during the next monsoon season, threatening the main embankment near the Manos River estuary. For this reason a new groyne shall be constructed 200 m downstream from the present groyne G-A. Its direction is pointing at 15° against the projects' main base line, i.e. is planned parallel to groyne G-A. The total length of the groyne is designed at 81.35 m and the structure is composed of 14 Nos. in-situ concrete bored piles dia. 914 mm (36") and 20 Nos. tubular steel piles, dia. 711.2 x 14.2 mm. The steel pile material will be delivered by the Consultant to the Contractor at the Port of Chittagong.
- (2) In-situ concrete bored piles are to be constructed in accordance with the Specifications, Section 1200. The length of tubular steel lining shall be 8.4 m for all piles.

Appropriate bored pile production reports are to be prepared by the Contractor for each pile in accordance with Subsection 1270 of the Specifications.

- (3) Construction of bored piles shall start from the toe of the present embankment, progressing towards river-side. As soon as the first six piles are completed, the improvement of the existent BWDB-embankment shall start within 50 m upstream and downstream from the center line of the new groyne, including connecting the new revetment to the existent one, which terminates about 150 m downstream from groyne G-A. Details are described in Chapter 2, para (2), (6) and (7) of this Description of Works. Toe protection and launching apron of cc-blocks are to be placed subsequently.
- (4) The start of installation of tubular steel piles depends on the timely arrival at Site of the material, which is expected by 15.03.96. In case of a major delay in the arrival of steel pile material, it may have to be optioned to continue construction of the new groyne by in-situ concrete bored piles as follows:

Axis 14 to 18: 4 bored piles dia. 914 mm, length 30 m, at 2.3 m spacing,

Axis 18 to 25: 7 bored piles dia. 914 mm, length 34 m, at 3.0 m spacing.

All such additional piles shall likewise receive a permanent steel tube lining of 8.4 m length.

Respective decision is to be made by the Consultant in consultation with the Contractor at the appropriate time.

- (5) Tubular steel piles are to be installed with piling equipment operating on the flood plain, but the last 3 or 4 (river-sided) piles will have to be installed by Consultant's floating piling equipment.

Due to limited leader length of bank-sided piling equipment, it will be required to install the piles in two to three sections:

Initially 12 m long sections shall be installed at the designed locations. Subsequently the pile heads are to be cut about 10 m below pile top (or below deformation, if any) and bevelled. The second pile section of 12 m is to be placed and to be well aligned with the pile axis of the installed first pile section. To ease the installation the pile extension sections should be provided with 3 to 4 guide brackets welded to the piles' interior. A full penetration butt weld is to be carried out, meeting the requirements as per Subsection 1531 of the Specifications.

Ceramic backing rings will be utilized to ensure proper root welding. The ceramic backing rings are supplied by the Consultant free of cost to the Contractor.

After inspection/testing of the completed butt welds the piles are to be driven to a level suitable to receive the last extension section. Dependent on the location, the section lengths vary from about 2 m to 8 m. The butt joint for the third extension is to be carried out as described above. Finally, the piles are to be driven to the designed head level. No further cutting of the pile heads to design level is required.

Finally, the completely installed piles shall be filled with soil to the top, as shown on the Drawings.

- (6) The tubular steel piles to be installed by the floating pile installation equipment shall be installed in full length, provided the site/bankline conditions permit to do so at the time of work execution. As



GR

the case may be, some dredging/ excavation into the flood plain may be required to carry out the pile installation.

- (7) Records are to be prepared for the complete pile installation process in accordance with Subsection 1560, Specifications. All piles have to reach the designed toe levels. In case of non-reaching the design depth, Subsection 1553 of the Specifications applies. All such measures are to be coordinated with the Consultant.
- (8) The most river-sided pile (No. 34) shall be topped with a "Danger Point Mark", which conical shaped top is to be painted "RED", using durable, preferably epoxy-resin based paint, after proper derusting and primer coating.

4 EXTENSION AND MODIFICATION OF GROYNES G-3 AND G-2

- (1) During the forthcoming monsoon season 1996 severer attack is expected at groyne G-3, wherefore modification and extension of this groyne shall be given priority over groyne G-2.

All works related to the extension and modification of groynes G-3 and G-2 are on the critical path due to relatively late arrival at Site of the newly supplied tubular steel pile material.

- (2) As stated under Chapter 2, para (1) a gap of about 30 m width shall be kept in the main embankment at the location of groynes G-3 and G-2, to facilitate pile installation works. The area of future pile installation works is to be levelled at a suitable elevation.

Stock-piles of soil for construction of the future groyne heads are to be prepared at the nearest point, refer to Chapter 2, para (2), last paragraph.

In addition all protection material, such as wire mesh, bricks for mattresses, the individual cc-block sizes and boulders are likewise to be kept ready at stock-piles, facilitating easy and speedy transport to the final location.

Should the newly imported steel piles arrive too late to facilitate completion of groynes G-3 and/or G-2, the gap in the embankment shall be closed and the river-sided slopes protected, utilizing the stock-piled materials. The deadline for this measure is to be coordinated with the Consultant.

- (3) Immediately after arrival of tubular steel pile material at Site and provided the remaining period of the construction window is considered adequate to complete these works, the on-shore piling shall commence at groyne G-3.

Dependent on the bankline condition at the time, it is expected that 10 Nos. of piles (location No. A18 to A9) dia. 711.2 x 14.2 mm (L = 24 m and 28 m) can be installed, commencing with the most river-sided one. Due to limited leader length of the piling equipment, it will be required to install the piles in two respectively three sections each, analogously to the procedures described under Chapter 3, para (5).

Subsequently the land-sided piling equipment is to be shifted to groyne G-2, where only about 4 piles, dia. 711.2 x 14.2 mm (L = 28 m) can be installed analogously to the above.

- (4) After the first few piles dia. 711.2 x 14.2 mm have been welded at the yard to designed length the nearest pile location which can be reached with the leader of the floating piling equipment at groyne G-3 shall be installed. It is expected that this may be pile No. 25 (dependent on riverbed situation at the time of construction).

After its installation, horizontal pile guide beams shall be arranged between pile Nos. A18 and A25, to facilitate installation of piles A19 to A24 (L=28 m to 32 m) without using the leader. These guides may be tubular steel piles dia. 711.2 mm supported near the heads of the said piles. The piles in the transition between the last land-sided one (A18) and the first river-sided driven one (A25) may have to be installed in sections analogously to the procedures described under Chapter 3, para (5). Initially by PTC-vibrator, but finally driving will have to be done using the Menck piling hammer. Respective measures are to be coordinated with the Consultant and the pile welding schedule for works at the yard is to be drawn up accordingly.

After completing the transition at groyne G-3, the floating pile driver has to move to groyne G-2, where the transition between the last land-sided pile and the first new one driven in the river shall be executed analogously to above procedures.

Only after completing the transition at groyne G-2, the floating pile driver shall complete the remaining pile installation works in the sequence G-3, G-2, G-A and G-A/2.

- (5) After completing the pile installation works at groynes G-3 and G-2, the pile head plates and bracings (G-3), respectively the original gangways (G-2) are to be installed. The gangway sections recovered from G-2 will not be adequate to cover the extended groyne length, but need to be supplemented by additional gangway sections, which will be of a simple design.
- (6) Immediately after completion of land-sided piling at groynes G-3 and G-2 the river bed profile in front of the future impermeable groyne heads has to be surveyed and the prevailing condition be compared with the profiles shown on the Drawings. If with the prevailing situation the groyne heads cannot be constructed with the designed slopes and toe levels, the length of the impermeable groyne is to be reduced and/or the excavation/ finishing level of the slopes to be adjusted, however, the slope inclination shall not be altered. The respective measures and decisions are to be coordinated with the Consultant.
- (7) The built-up of the impermeable groyne heads must start accordingly to the results of para (6) above, utilizing material stock-piled for that purpose. Note must be taken that during piling works for the transition to the river-sided pile structure slides are likely to occur near the bank, wherefore final shaping of groyne heads should be done only thereafter.

The slope of the groyne heads has to be shaped smoothly from 1:3 at the main embankment to 1:5 along the main axis of the groyne. Along the groynes' 1:5 slope a 1 m high step has to be integrated by shifting the center of the upstream and downstream slopes 5 m apart. The step itself is to be constructed by cc-blocks, size 50/50/50 cm, laid on well compacted surface and geotextile filter mat, in block masonry pattern.

At groyne G-3 the step is located just upstream from the groynes' pile structure, but at groyne G-2 the axis of the impermeable groyne is designed 5 m upstream from the axis of the pile structure.

The head of impermeable groynes is to be protected by cc-blocks, size 30/30/30 cm, hand-laid in block pattern on geotextile filter mat. Along the horizontal rows a 10 mm wide gap has to be arranged between the blocks to permit seepage. The transitions to the main embankment are to be protected by 20 cm thick brick mattresses, likewise to be laid on geotextile filter materials.

The launching aprons around these groyne heads are to be laid at the designed levels, refer to para (6) above.

- (8) Should for any reason (e.g. late arrival of piles or severely progressing changes of bankline and river bed) the construction of the impermeable groyne heads be impossible within the remaining period of time, the gaps left in the main embankment at groynes G-3 and G-2 are to be closed and the river-sided slopes be protected with 15 cm thick brick mattresses, to be laid on geotextile filter mats. The respective deadlines and measures are to be coordinated and agreed upon with the Consultant.

5 PROVISION OF FALLING APRONS AROUND THE PILE STRUCTURE OF GROYNES G-A AND G-2

(Drawings AD-KA-020, -040)

- (1) Based on the assumption that groyne G-A will be subjected to severe river attack, it is important to complete the falling apron around the pile structure as early as possible.
Material shall be cc-blocks $D_n = 35$ cm and/or boulders of size $D_{50} = 35$ cm, which depends on the timely availability.

Dumping of protection material shall start from the river-side moving towards bank and from downstream towards upstream. It is to be ensured that the designated quantity of material is reasonably uniformly distributed within the defined area. Dependent on the river flow situation at the time of placing, special consideration must be given to the fact that dumped material will considerably be displaced by the current flow wherefore dumping locations are to be decided accordingly.

Material dumping can fully be completed on the downstream side of the groyne, but on the upstream side only after complete installation of all supplementary piles.

- (2) Upon completion of all pile installation works for modification and extension of groyne G-2, the falling apron is to be installed around the newly driven piles, as shown on the Drawings.

Material shall be cc-blocks $D_n = 35$ cm and/or boulders of size $D_{50} = 35$ cm, which depends on the timely availability of the one or the other. Placing shall follow the procedures stipulated under above para (1).

It is to be noted that the installation of the supplementary falling apron at groyne G-2 has a relative low priority. In case of material shortage at the time, it may be optioned to postpone or cancel this measure. Respective decision is to be arrived at in consultation with the Consultant.

6 PROTECTION MEASURES TO THE SHEET-PILE COFFERDAMS AT GROYNES G-A AND G-1

(See Drawings AD-KA-040, -010)

- (1) With the increasing river attack at groyne G-A, where the bank has reached practically the river-sided wall of the reinforced concrete sheet pile cofferdam (January 1996), additional measures were initiated early January 1996 by dumping cc-blocks and boulders of size 30 cm and above around the cofferdam head, covering an area with a radius of about 20 m. Due to insufficient quantity of material the work could not be completed, wherefore additional protection material (cc-blocks or boulders of size 30 cm and above) are to be placed to achieve the full coverage of 2,500 Nos. per 100 m² of protection area.

In addition, the scour protection at the downstream side of the cofferdam shall be extended to 20 m width by dumping cc-blocks and/or boulders of size 30 cm and above, at a cover rate of 2,500 Nos. per 100 m² area.

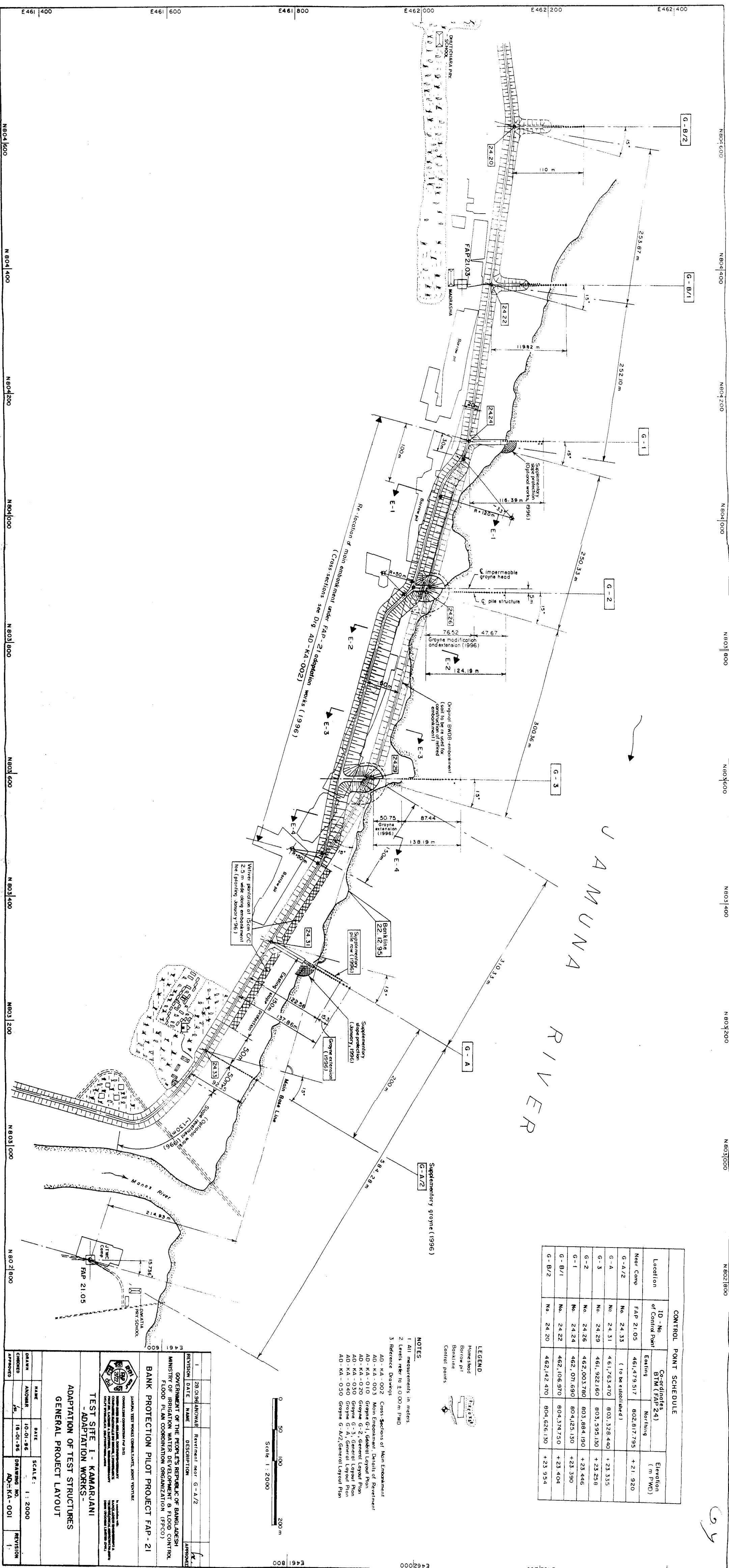
The choice of material depends on its timely availability and is to be decided at Site.

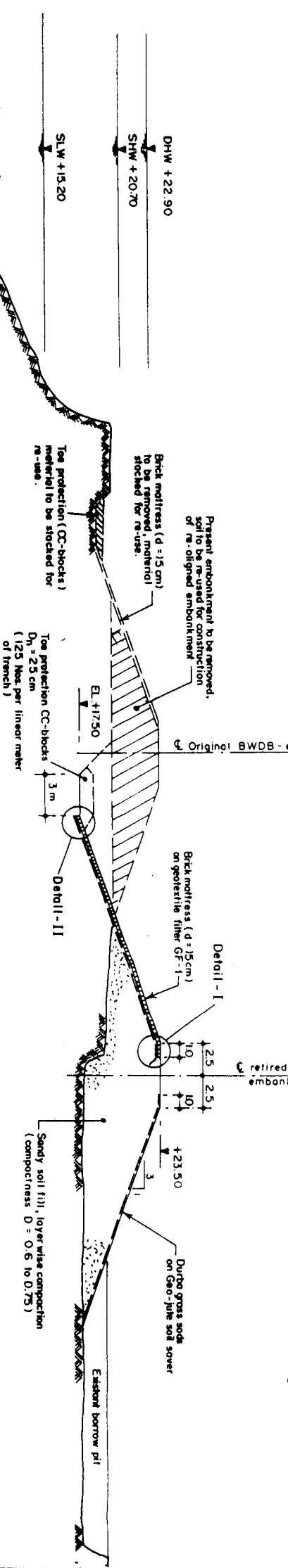
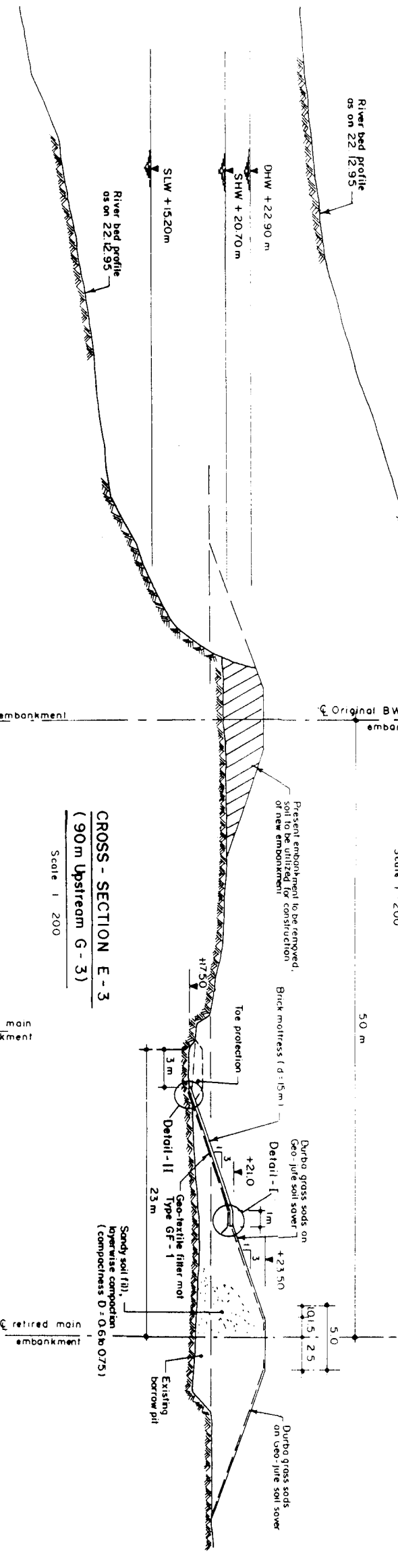
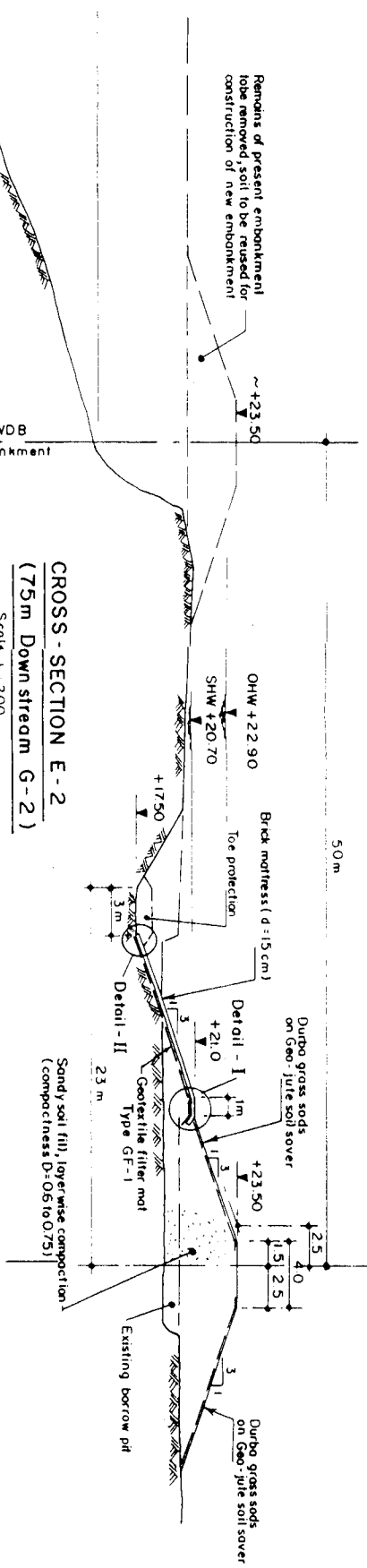
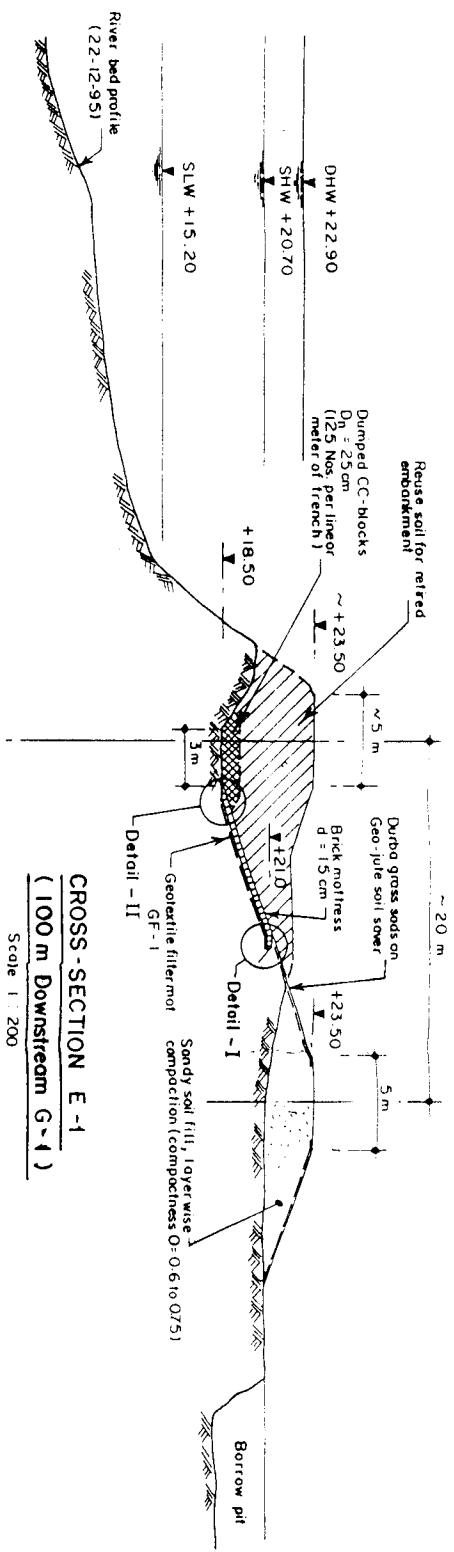
- (2) The steel sheet pile cofferdam of groyne G-1 is exposed at its downstream edge. With the situation January 1996 there is no immediate threat to the structure, the more since no severe attack to this groyne is expected during the coming monsoon season as per latest morphological developments in the test site area.

Should time permit and surplus protection material, either cc-blocks or boulders of size 30 cm and above, be available at the time, additional protection material is to be dumped, covering an area with a radius of about 15 m around the downstream edge of the cofferdam.

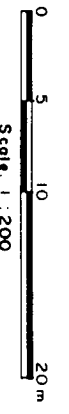
The respective measures are to be coordinated with the Consultant, based on up to date soundings in the respective area.

Areas within the bed protection around the pile structure where the filter layer (geo-jute sand mat) is presently exposed, shall adequately be covered by boulders of size 30 cm or above.





- NOTES**
1. All measurements in meter.
 2. Levels refer to ± 0.0 m PWD.
 3. S.L.W. = Standard Low Water
 4. D.H.W. = Design High Water
 5. Details of revetment see Dwg. AD-KA-003
- Reference Drawings:**
 AD-KA-001 Adoption of Test Structures
 AD-KA-003 Main Embankment, Details of Revetment
 AD-KA-020 Groynes G-2
 AD-KA-030 Groynes G-3
 AD-KA-040 Groynes G-4
 General Project Layout



DRAWING PHOTOREDUCED BY 50%

REV	DATE	NAME	DESCRIPTION	APPROVED

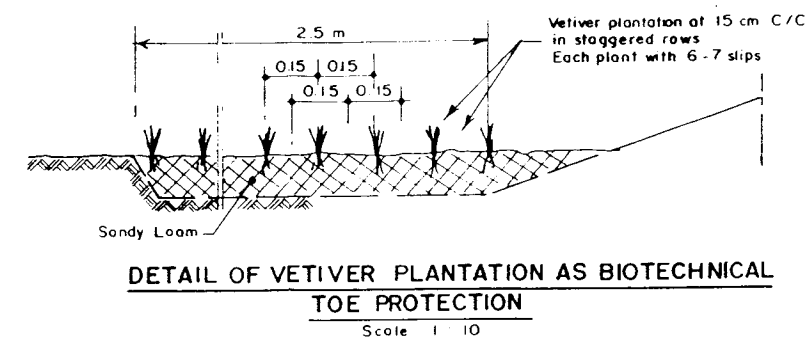
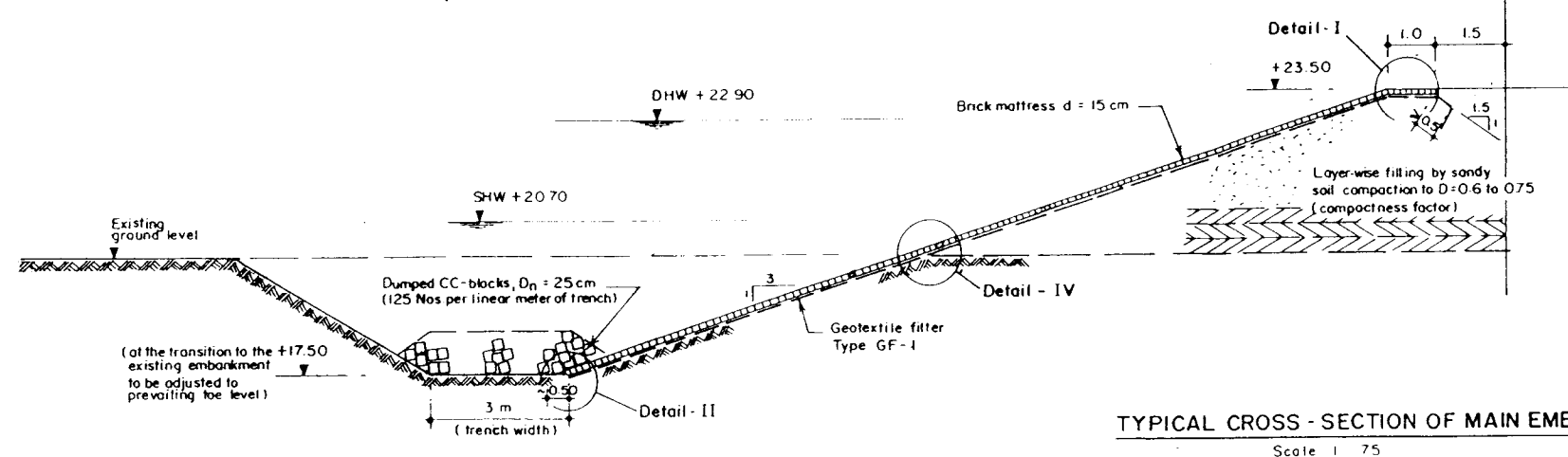
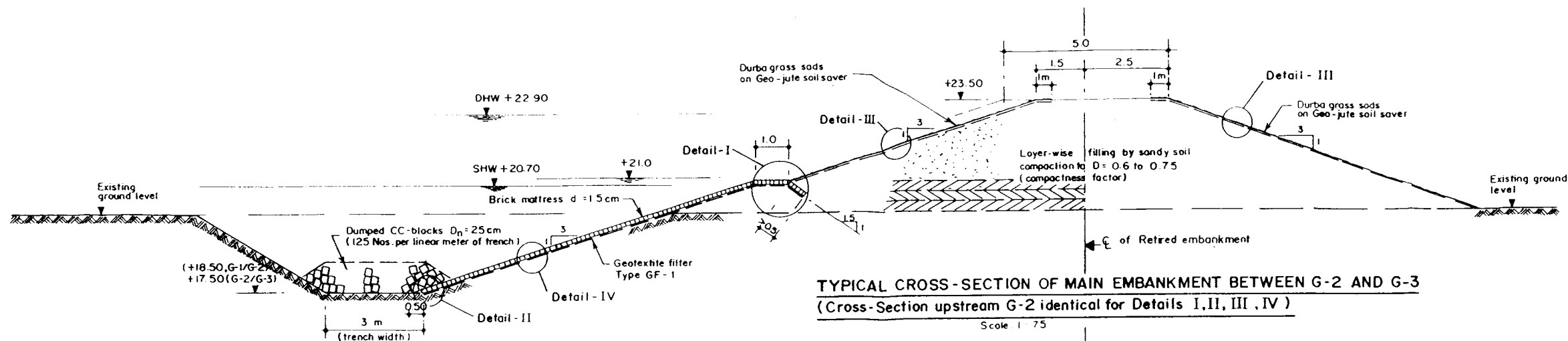
GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH
MINISTRY OF IRRIGATION WATER DEVELOPMENT & FLOOD CONTROL
FLOOD PLAN COORDINATION ORGANIZATION (FPCCO)

BANK PROTECTION PILOT PROJECT FAP-21

TEST SITE I - KAMARJANI
- ADAPTATION WORKS -

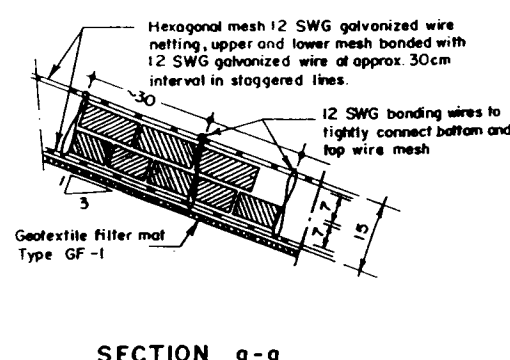
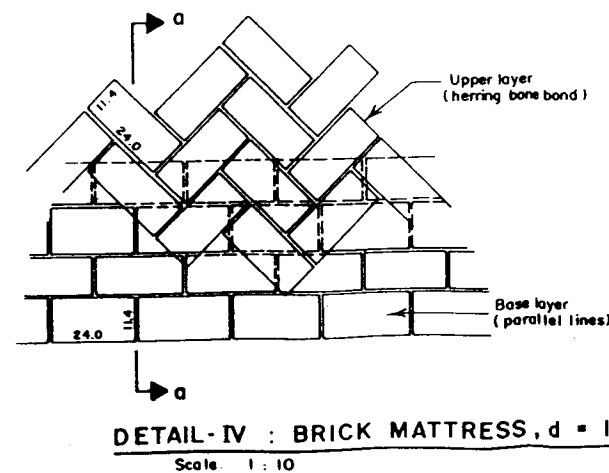
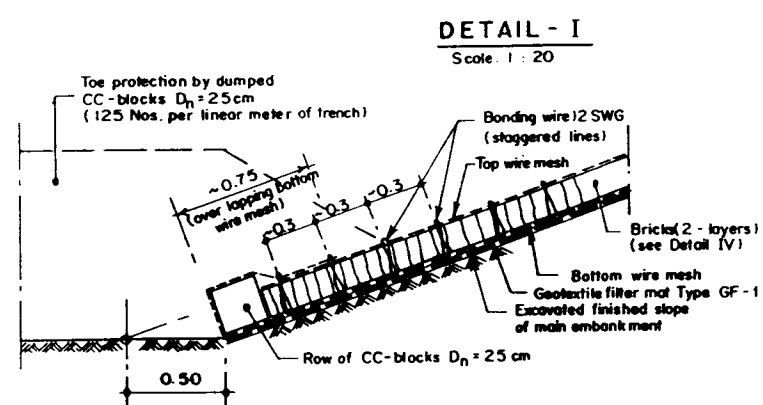
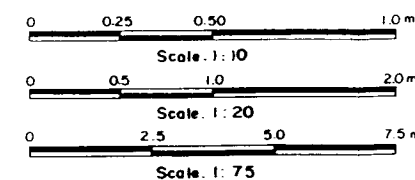
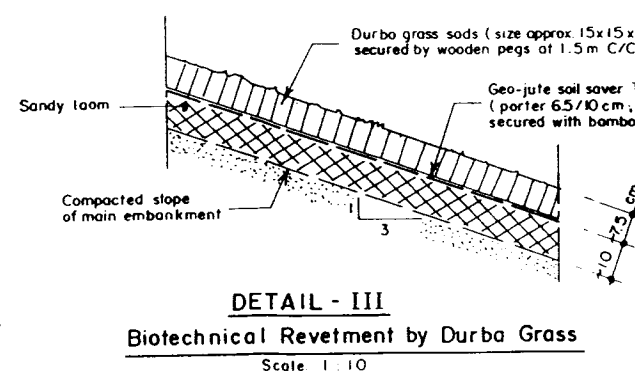
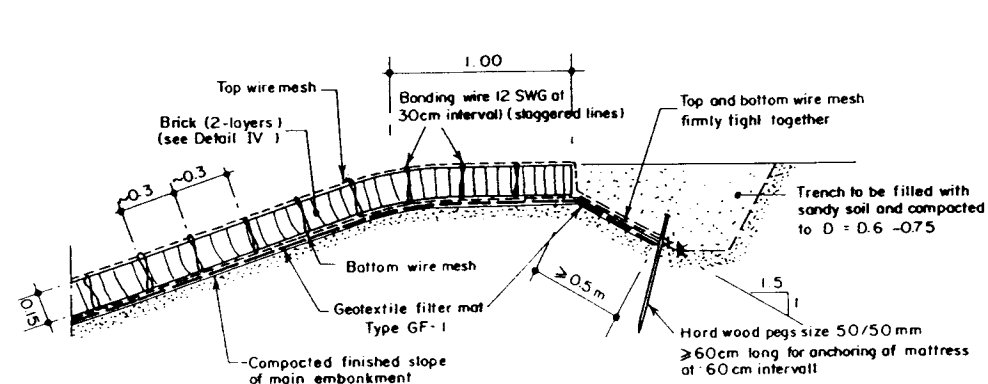
CROSS-SECTION OF MAIN EMBANKMENT
(E-1 to E-4)

NAME	DATE	SCALE	DRAWING NO.	REVISION
DR. AMIN	17-01-96	1 : 200	AD-KA-002	0
CHECKED	22-01-96			
APPROVED				



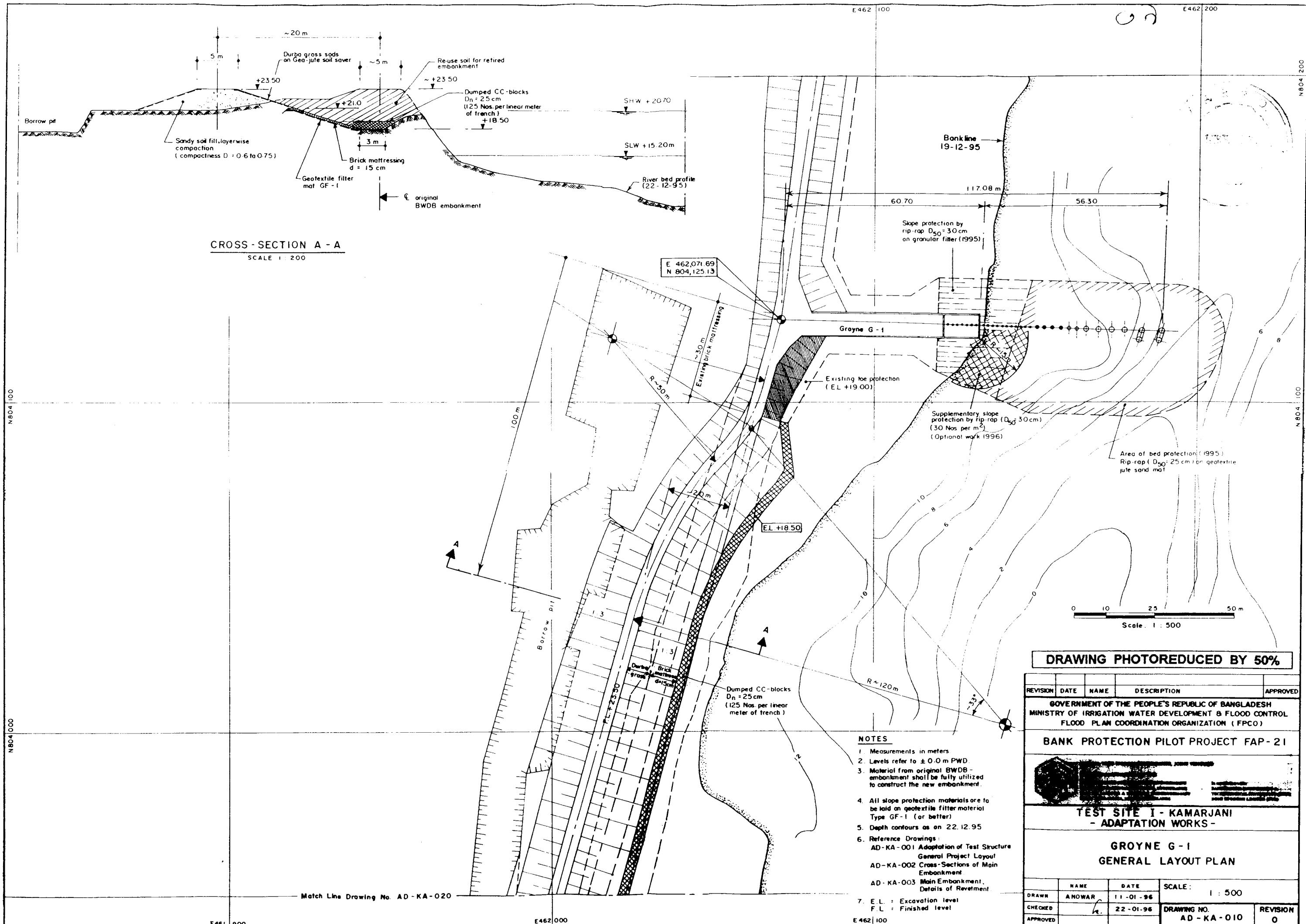
NOTES

1. Measurements in meters unless shown different.
2. Levels refer to ± 0.0 m PWD.
3. Filling and compaction of sandy soil as per Specifications, Section 900.
4. Filter materials and protection materials and their installation as per Specification Section 1000.
5. Reference Drawings:
AD-KA-002 Cross-Sections of Main Embankment.

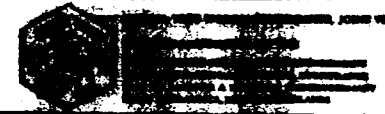


DRAWING PHOTOREduced BY 50%

REV.	DATED	NAME	DESCRIPTION	APPROVED
GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH MINISTRY OF IRRIGATION WATER DEVELOPMENT & FLOOD CONTROL FLOOD PLAN COORDINATION ORGANIZATION (FPCO)				
BANK PROTECTION PILOT PROJECT FAP-21				
TEST SITE I - KAMARJANI - ADAPTATION WORKS -				
MAIN EMBANKMENT DETAILS OF REVETMENT				
DRAWN	NAME	DATE	SCALE:	
CHECKED	ANOWAR	18-01-96	1:10, 1:20, 1:75	
APPROVED		23-01-96	DRAWING NO.	REVISION
			AD-KA-003	0



DRAWING PHOTOREduced BY 50%

REVISION	DATE	NAME	DESCRIPTION	APPROVED
GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH MINISTRY OF IRRIGATION WATER DEVELOPMENT & FLOOD CONTROL FLOOD PLAN COORDINATION ORGANIZATION (FPCO)				
BANK PROTECTION PILOT PROJECT FAP-21				
				
TEST SITE I - KAMARJANI				
- ADAPTATION WORKS -				
GROYNE G-1				
GENERAL LAYOUT PLAN				
			SCALE:	1 : 500
DRAWN	ANOWAR	11-01-96	DRAWING NO.	AD-KA-010
CHECKED		22-01-96	REVISION	0
APPROVED				

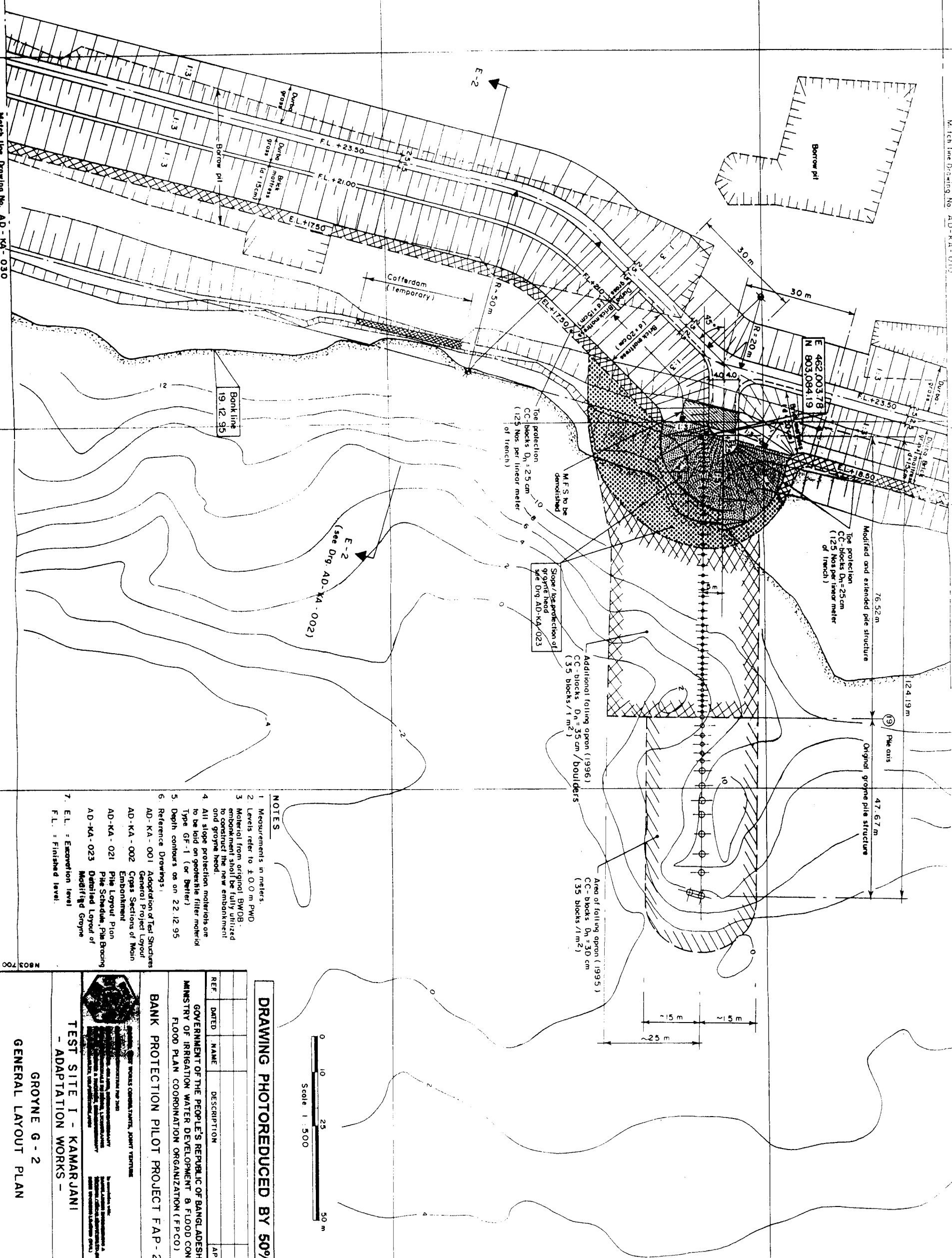
E 461 900

E 462 000

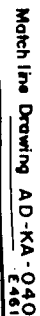
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Match line Drawing No. AD-KA-010

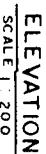
86



N 803 500



CHECKED	by	22-01-96	DRAWING NO.	REVISION
APPROVED			AD-KA-030	0



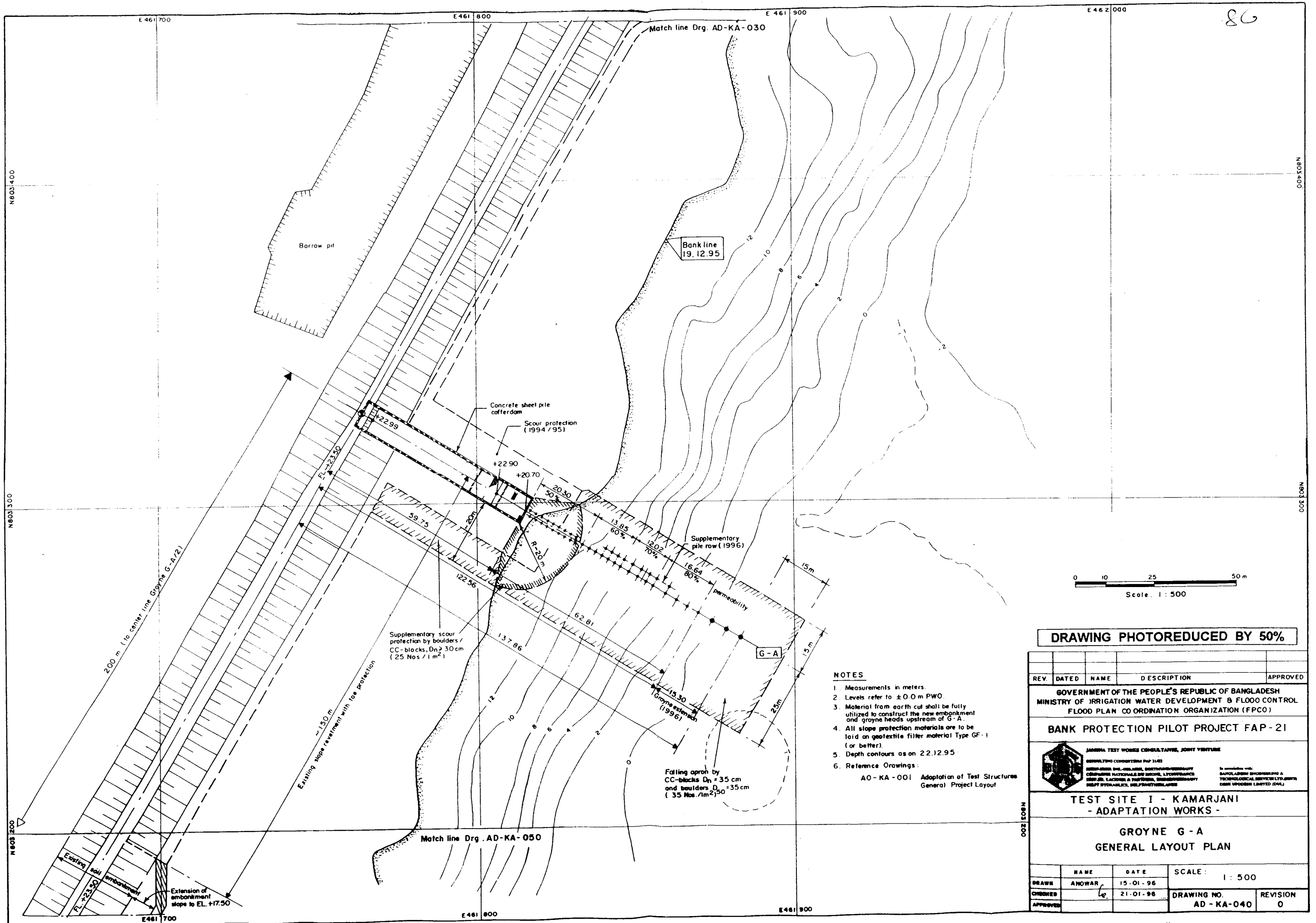
DETAIL A
SCALE 1:20

- 1 All measurements in meters
- 2 All levels refer to ± 0 Comp WD
- 3 S L W = Standard Low Water
- 4 H W = Design High Water
- 4 Reference Drawings
- KA-305/3 Pile Installation (Pen (as built))
- AD-KA-032 Steel Pile Dia 7 ft x 14.2 mm
- Pile Schedule, Detail
- AD-KA-033 Detailed Layout of Modified Groyne

BANK PROTECTION PILOT PROJECT FAP-21

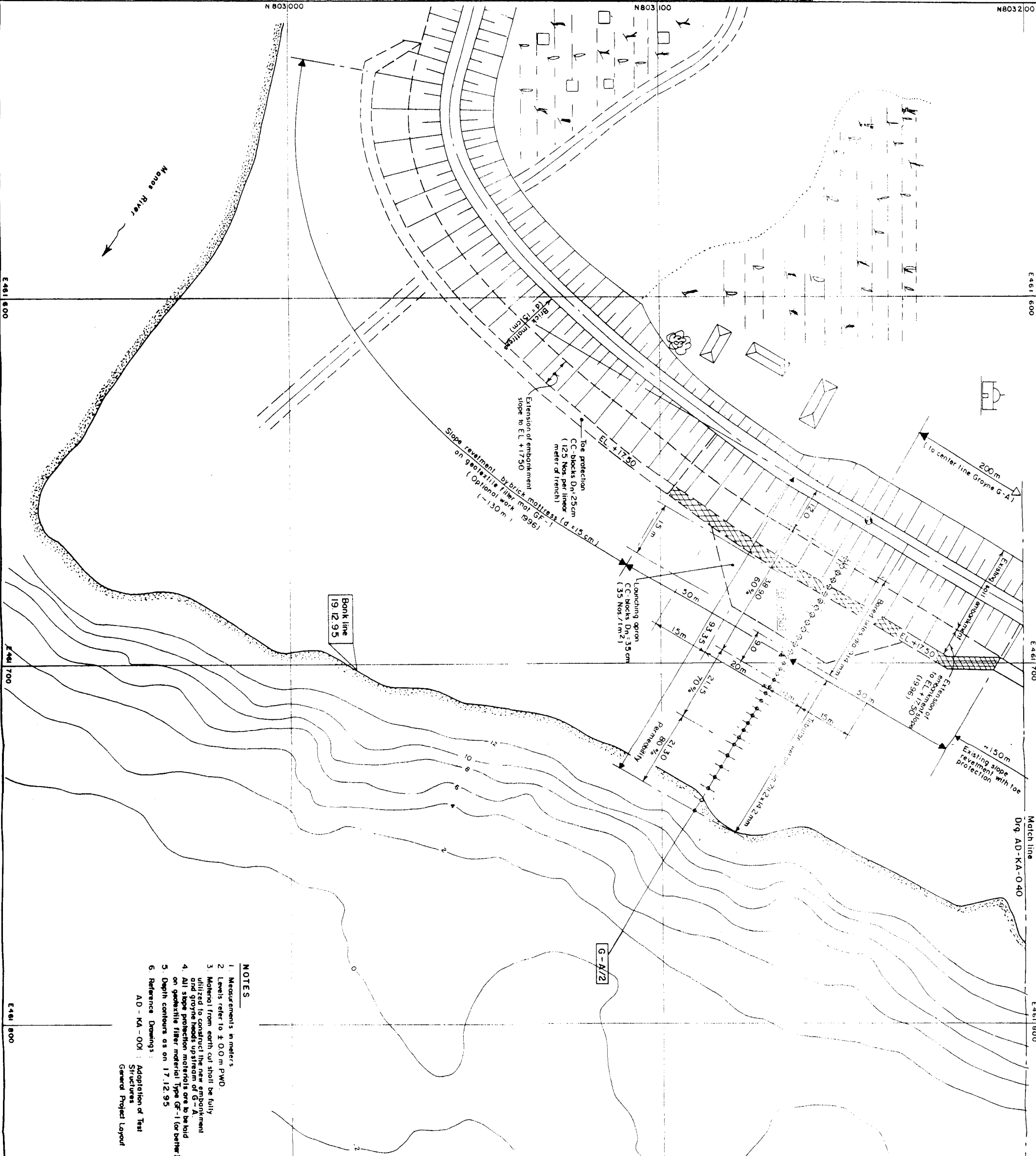
PILE LAYOUT PLAN
PILE SCHEDULE--PILE BRACING

NAME	DATE	SCALE: 1:200, 1:20
DR/WR	F. Hesse	
CHECKED	10-01-96	DRAWING NO. AD-KA-031
APPROVED		REVISION 0



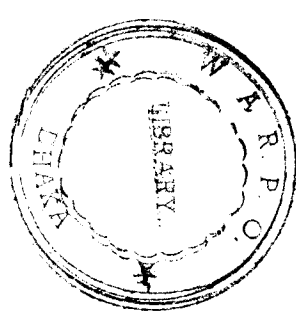
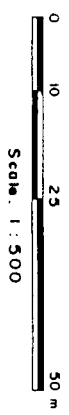
DRAWING PHOTOREduced BY 50%

REV.	DATED	NAME	DESCRIPTION	APPROVED
GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH MINISTRY OF IRRIGATION WATER DEVELOPMENT & FLOOD CONTROL FLOOD PLAN COORDINATION ORGANIZATION (FPCO)				
BANK PROTECTION PILOT PROJECT FAP-21				
JAMUNA TEST WORKS CONSULTANTS, JOINT VENTURE CONSULTING ENGINEERS PWP 1142 CONSULTING ENGINEERS, BANGKOK, THAILAND CONSULTING ENGINEERS, BANGKOK, THAILAND CONSULTING ENGINEERS, BANGKOK, THAILAND CONSULTING ENGINEERS, BANGKOK, THAILAND				
TEST SITE I - KAMARJANI				
- ADAPTATION WORKS -				
GROYNE G - A				
GENERAL LAYOUT PLAN				
DRAWN	NAME	DATE	SCALE	
ANWAR		15-01-96	1 : 500	
CHECKED		21-01-96		
APPROVED				
DRAWING NO.			AD - KA-040	REVISION 0



- NOTES**
1. Measurements in meters.
 2. Levels refer to ± 0.0 m PWD.
 3. Material from earth cut shall be fully utilized to construct the new embankment and groyne heads upstream of G-A.
 4. All slope protection materials are to be laid on geotextile filter material Type GF-1 (or better).
 5. Depth contours as on 17.12.95.
 6. Reference Drawings:
AD - KA - 001 : Adaptation of Test Structures
General Project Layout

DRAWING PHOTOREDUCTION BY 50%



REV.	DATED	NAME	DESCRIPTION	APPROVED
1	28.1.96	Anowar	Launching apron	

GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH
MINISTRY OF IRRIGATION WATER DEVELOPMENT & FLOOD CONTROL
FLOOD PLAN CO-ORDINATION ORGANIZATION (FPDO)

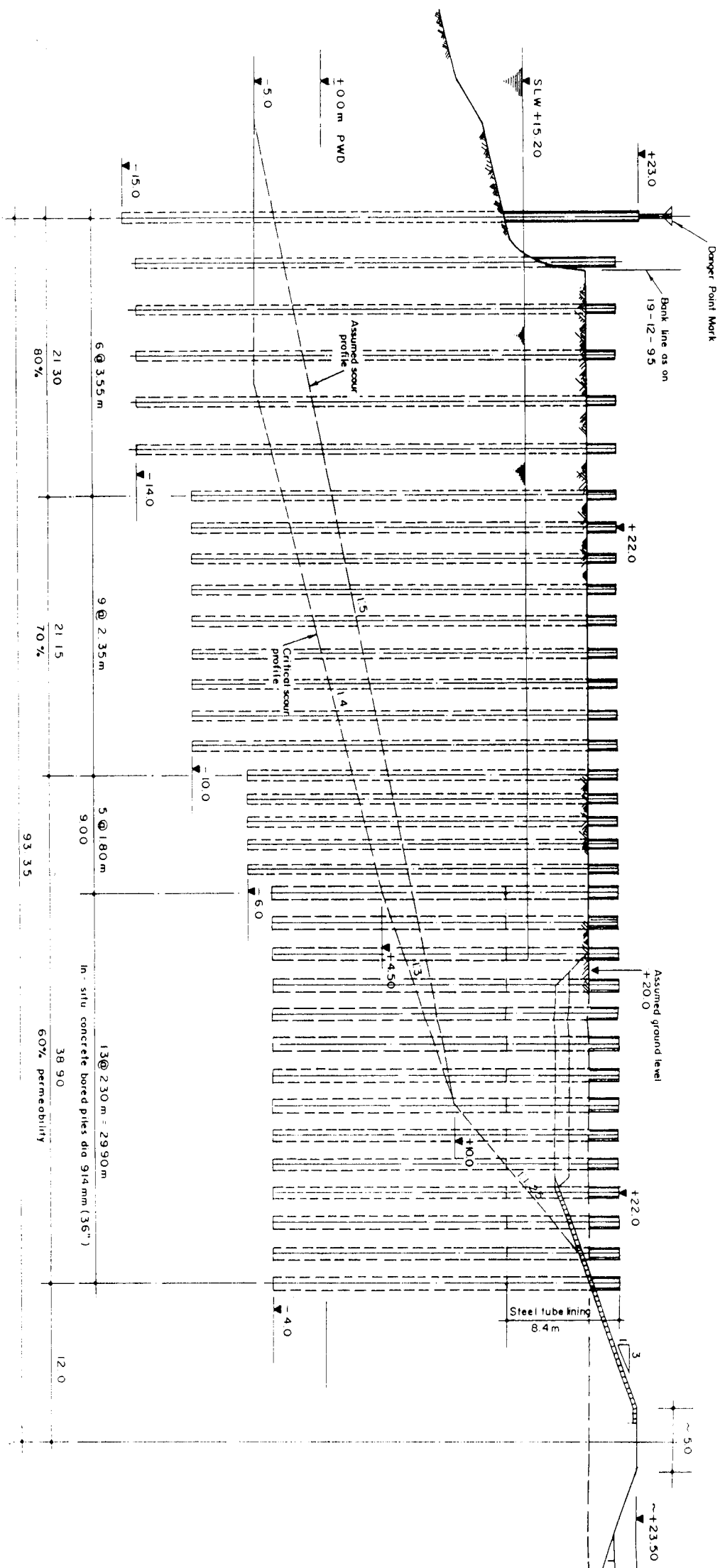
BANK PROTECTION PILOT PROJECT FAP-21

TEST SITE I - KAMARJANI
- ADAPTATION WORKS -

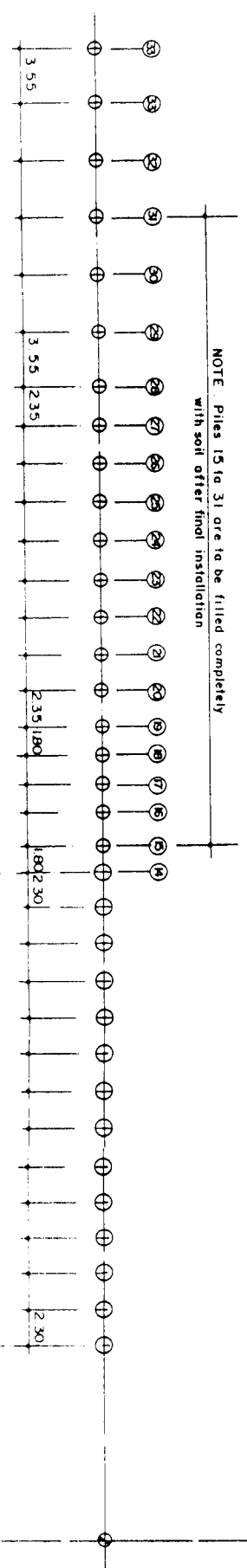
GROYNE G-A/2
GENERAL LAYOUT PLAN

NAME	DATE	SCALE
ANOWAR	18.01.96	1:500

DRAWING NO.	REVISION
AD-KA-050	1



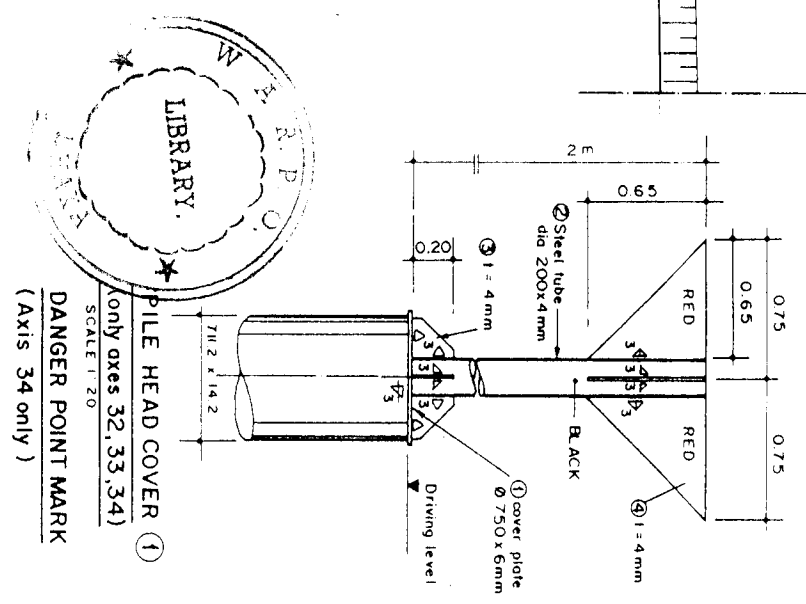
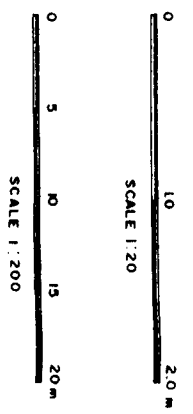
ELEVATION
SCALE 1:200



PLAN
SCALE 1:200

PILE INSTALLATION SCHEDULE				
PILE TYPE	LOCATION NO.	PILE HEAD LEVEL m PWD	PILE POINT LEVEL m PWD	PILE LENGTH m
Bored pile dia 914 mm	1 to 14	+22.0	-4.00	26.0
Steel pile dia 2016 x 711.2 x 14.2 mm	15 to 19	+22.0	-6.00	28.0
Steel pile dia 2016 x 711.2 x 14.2 mm	20 to 28	+22.0	-10.00	32.0
Steel pile dia 2016 x 711.2 x 14.2 mm	29 to 33	+22.0	-14.00	36.0
Steel pile dia 2016 x 711.2 x 14.2 mm	34	+23.0	-15.00	38.0

- NOTES:
- All measurements in meters
 - Levels refer to 100.0 m PWD
 - SLW = Standard Low Water
 - Reference Drawings
 - AD-KA-001 Adoption of Test Structures
 - AD-KA-050 General Project Layout
 - AD-KA-052 Steel Pile Dia 711.2 x 14.2 mm Pile Schedule



MATERIAL SCHEDULE				
ITEM NO.	SECTION / SIZE	UNIT WEIGHT (kg)	WEIGHT (kg)	
1	3	750 x 6	26.5 / No	79.5
2	1	200 x 4	19.3 / m	38.6
3	4	200 x 4	1.25 / No	5.0
4	4	650 x 4	6.63 / No	26.5
TOTAL				149.6

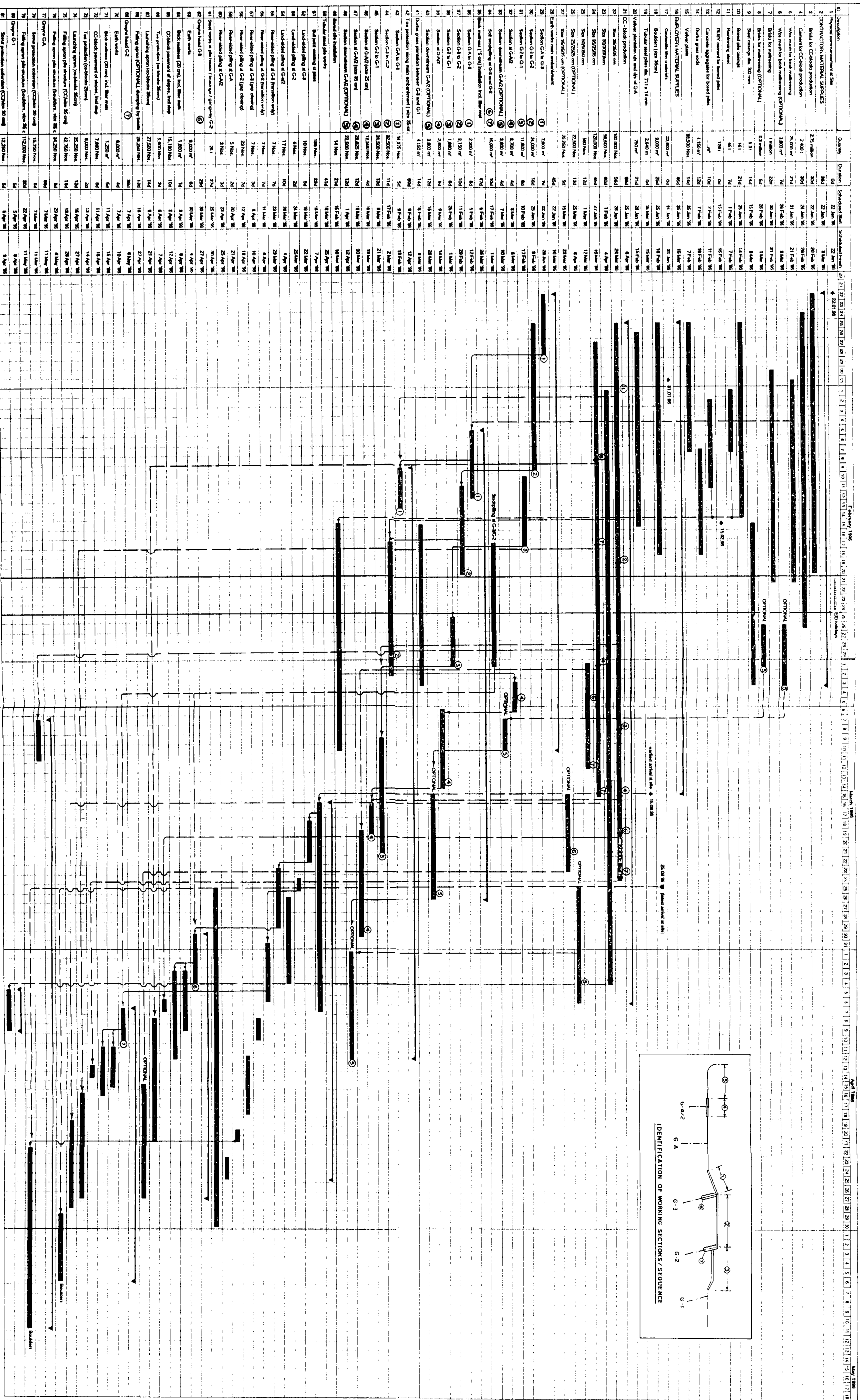
DRAWING PHOTOREDUCTION BY 50%

REV	DATE	NAME	DESCRIPTION	APPROVED
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GOVERNMENT OF THE PEOPLE'S REPUBLIC OF BANGLADESH MINISTRY OF IRRIGATION, WATER DEVELOPMENT & FLOOD CONTROL FLOOD PLAN COORDINATION ORGANIZATION (FPCCO)				
BANK PROTECTION PILOT PROJECT FAP-21				
JAMUNA WEST BRIDGE GENERAL PART, JOINT VENTURE				
TEST SITE 1 - KAMARJANI				
ADAPTATION WORKS -				
GROUPE G-A/2				
PILE LAYOUT PLAN				
PILE INSTALLATION SCHEDULE				
DANGER POINT MARK				
NAME	DATE	SCALE	1:200, 1:20	
DR. M. A. H. K.	20-01-96			
CHECKED				
APPROVED				
DRAWING NO.			AD-KA-051	
REVISION			0	

ANNEX G

**Detailed Construction
Time Schedule**

TEST SITE 1 - KAMARJANI
ADAPTATION OF TEST STRUCTURES (1996)
Detailed Construction Time Schedule

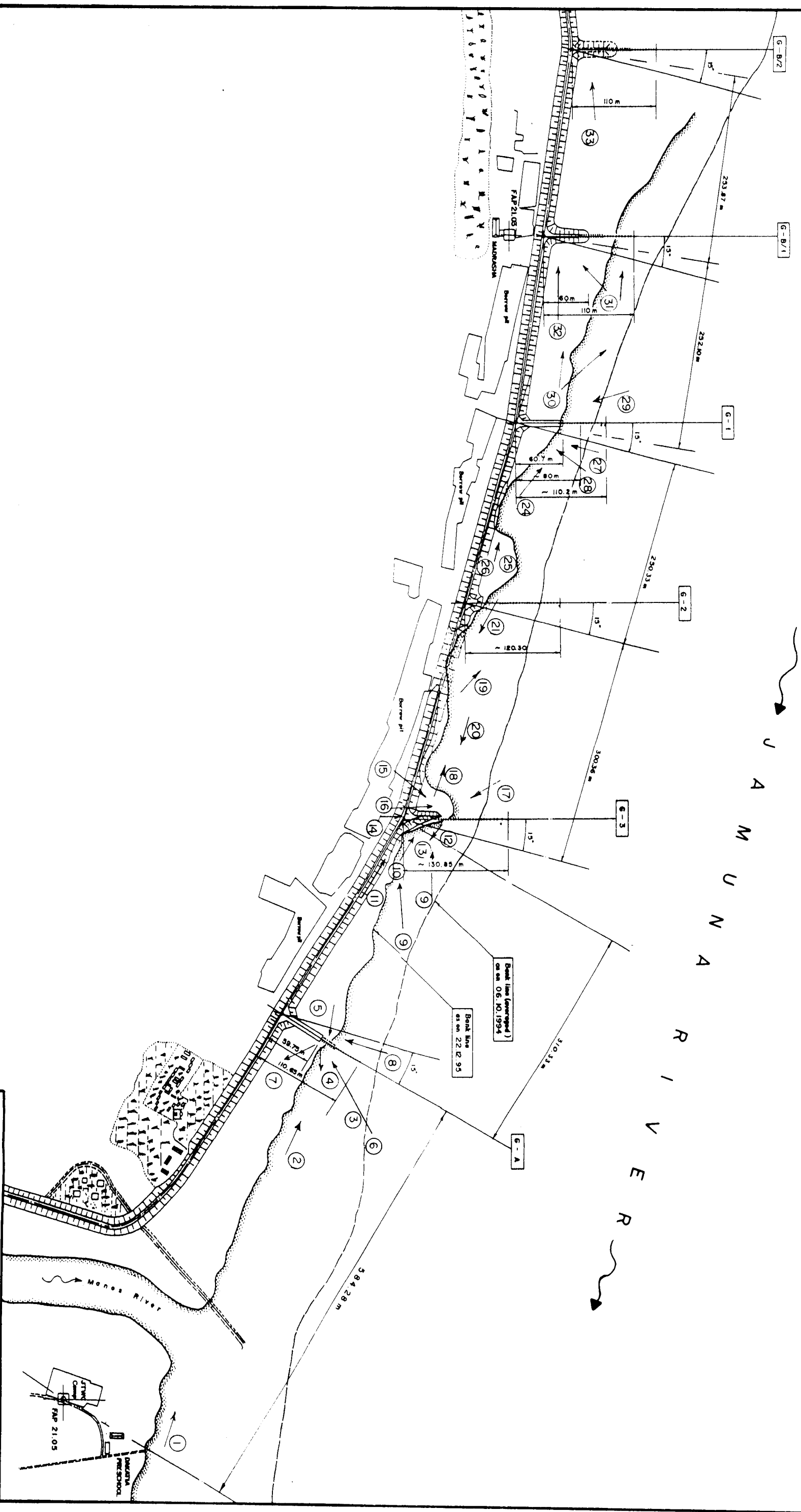


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ANNEX H

Photographs



TEST SITE I - KAMARJANI
SITUATION AFTER MONSOON 1995

Scale 1:5,000



Photo 1: Manos river estuary on 02.12.1995
(water level at 15.79 m PWD)



Photo 2: Groyne G-A; view from downstream on 02.12.1995
(water level at 15.79 m PWD)



Photo 3: Groyne G-A on 02.12.1995; bank erosion reached d/s edge of cofferdam (water level at 15.79 m PWD)



Photo 4: Groyne G-A on 02.12.1995; Falling apron near d/s edge of cofferdam (water level at 15.79 m PWD)

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Photo 5: Groyne G-A on 02.12.1995; erosion process around spun cc-piles (water level at 15.79 m PWD)



Photo 6: Groyne G-A on 03.02.1996; eroding bank reached d/s edge of cofferdam. Falling apron around cofferdam head completely unfunctional (water level at 14.48 m PWD)



Photo 7: Groyne G-A on 02.12.1995; view towards d/s main embankment (water level at 15.79 m PWD)



Photo 8: Groyne G-A on 02.12.1995; view along u/s side of cofferdam, no damages (water level at 15.79 m PWD)

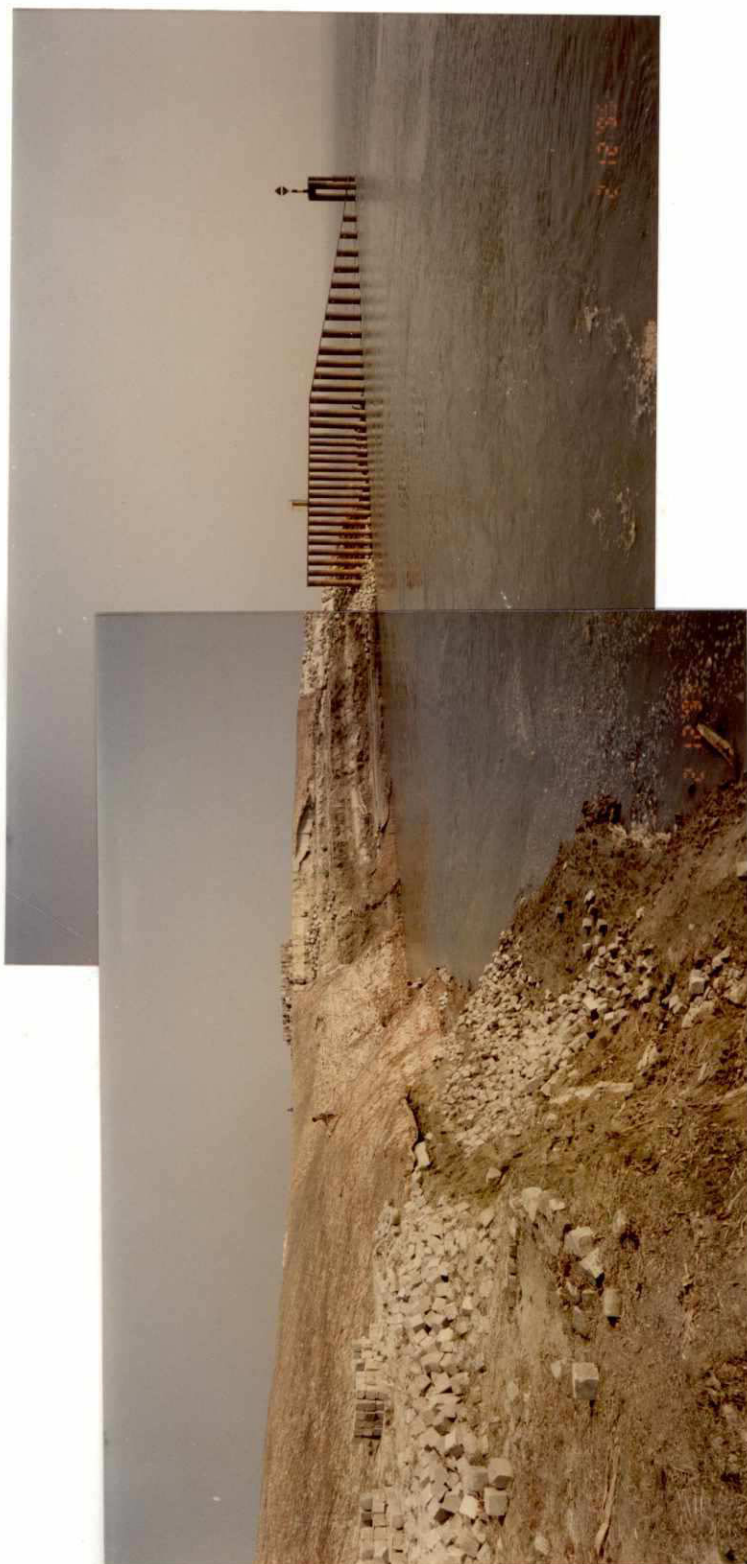


Photo 9: Groyne G-3 on 02.12.1995; view from d/s into damaged part of the groyne (water level at 15.79 m PWD)

22

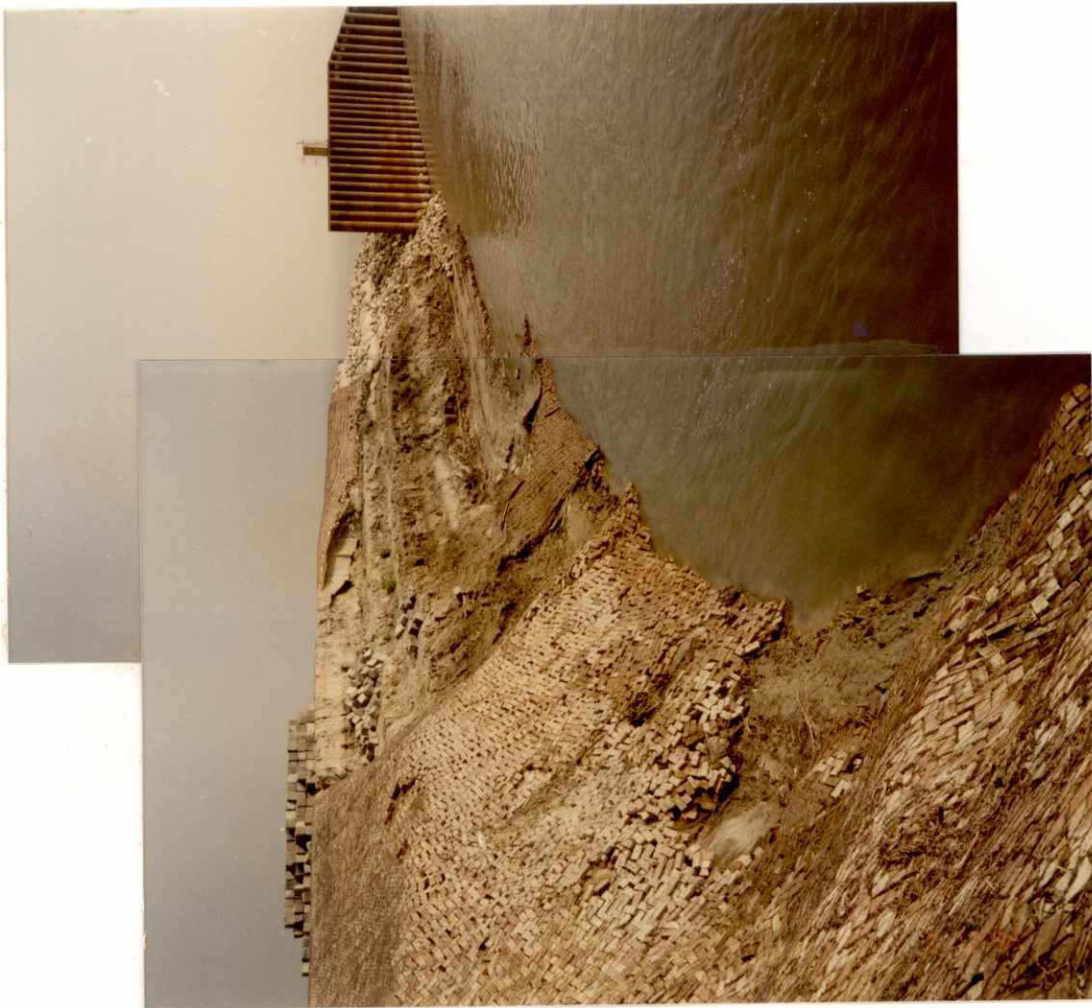


Photo 10: Groyne G-3 on 02.12.1995; view from d/s to damaged part of the groyne
(water level at 15.79 m PWD)

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Photo 11: Groyne G-3 on 02.12.95; Anchorage of brick mattress effected by major slide but yet functional (water level at 15.79 m PWD)



Photo 12: Groyne G-3 on 02.12.1995; view from groyne head towards downstream (water level at 15.79 m PWD)



Photo 13: Groyne G-3 on 02.12.1995; Damaged main embankment d/s from G-3 (water level at 15.79 m PWD)



Photo 14: Groyne G-3 on 02.12.1995; View along impermeable part of G-3; upstream portion yet fully in tact (water level at 15.79 m PWD)



Photo 15: Groyne G-3 on 02.12.1995; falling apron of u/s impermeable groyne head; stone dumping along piles likely to be excessive dumping during construction (water level at 15.79 m PWD)



Photo 16: Groyne G-3 on 02.12.1995; view along pile structure (u/s side) (water level at 15.79 m PWD)

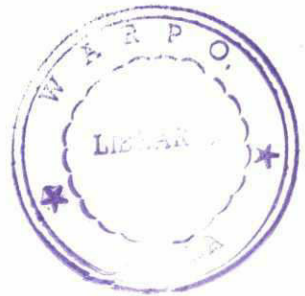




Photo 17: Groyne G-3 on 02.12.1995; (u/s side) scattered cc-blocks of falling apron (protection to impermeable groyne head) (water level at 15.79 m PWD)



Photo 18: Groyne G-2 on 02.12.1995; view from G-3 towards G-2 (water level at 15.79 m PWD)



Photo 19: Groyne G-2 on 02.12.1995; Damaged main embankment (water level at 15.79 m PWD)



Photo 20: Groyne G-2 on 02.12.1995; Temporary cofferdam approx. 80 m downstream from G-2 as closure for breached main embankment (water level at 15.79 m PWD)



Photo 21: Groyne G-2 on 02.12.1995; view from groyne toward (d/s) main damage to embankment (water level at 15.79 m PWD)



Photo 22: Groyne G-2; Tubular steel piles dia. 711.2x14.2 mm recovered from damaged part of groyne

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Photo 23: Groyne G-2; Tubular steel pile dia. 711.2x14.2 mm; material failure at max. deformation point about 6 m from pile tip



Photo 24: Groyne G-1 on 03.12.1996; view from downstream (water level at 15.75 m PWD)



Photo 25: Groyne G-1 on 03.12.1995; main embankment eroded by waves (Durba grass planted too late to develop proper rooting) (water level at 15.75 m PWD)



Photo 26: Groyne G-1 on 03.12.1995; main embankment eroded by waves and return current (water level at 15.75 m PWD)



Photo 27: Groyne G-1 on 03.02.1996; view from d/s side; slope protection functional (water level at 14.48 m PWD)



Photo 28: Groyne G-1 on 03.12.1995; slope deformation, rip-rap on granular filter layer (water level at 15.75 m PWD)



Photo 29: Groyne G-1 on 03.02.1996; View from u/s side; slope protection in tact; Geo-jute-sand mat partly exposed within bed protection around the piles (water level at 14.48 m PWD)

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Photo 30: View from G-1 toward groyne G-B/1 on 03.12.1995
(water level at 15.75 m PWD)

cf

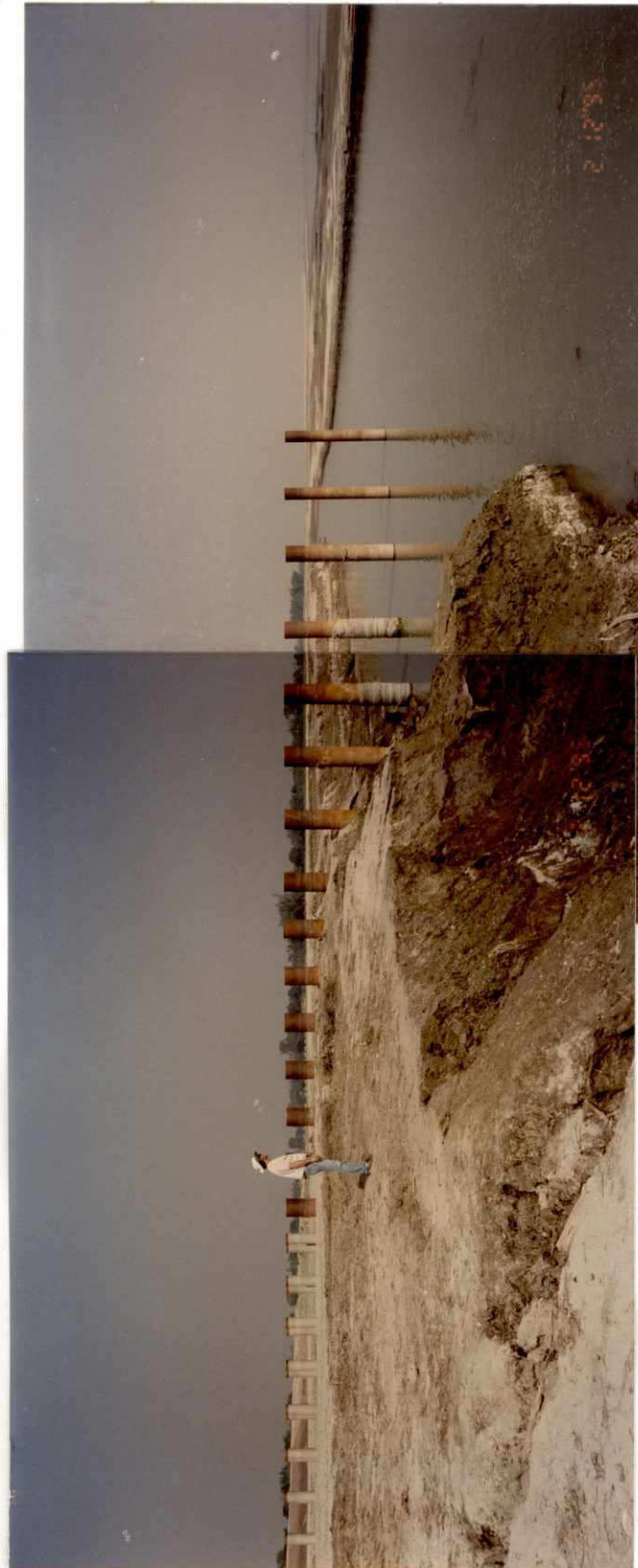


Photo 31: Groyne G-B/1 on 03.12.1995; pile structure partly exposed by on-going erosion (water level at 15.75 m PWD)



Photo 32: Groyne G-B/1 on 03.12.1995; Impermeable groyne without any damage



Photo 33: Groyne G-B/2 on 03.12.1995; Impermeable groyne; earthdam and Durba grass cover; downstream side eroded (Durba grass sods planted too late)

